

CHARLOTTE DOUGLAS INTERNATIONAL AIRPORT CHARLOTTE, NORTH CAROLINA

ADDENDUM NO. 1

NORTH END AROUND TAXIWAY (NEAT) OVERLOOK RELOCATION

CLT PROJECT NO. AVIA 23-50 ADDENDUM DATE: MAY 16, 2023

This Addendum is hereby made a part of the contract documents and specifications of the above referenced project. All other requirements of the original plans and specification shall remain in effect in their respective order. Acknowledge receipt of this addendum by initialing next to its number on the "Execution of Bid" page of your Bid.

Addendum No. 1 Includes the Following:

- Bidder Questions and Responses (as of 05.15.2023 at 10:00AM)
 Responses are in the color red.
- Project Manual (Front End) Revisions
- Technical Specification Revisions
- Line Item Revisions
- Plan Sheet Revisions
- Revised Section III. Bid Form and Supplements
 - o The revised bid form reflects the adjustments made to the line times
 - **NOTE**: Bid submissions must have the revised bid form included in this Addendum No. 1.
- Geotechnical Report
- Pre-Bid Meeting Attendance Sheet
- Pre-Bid Meeting PowerPoint Slides
- Plan Holders List (as of 05.15.2023)
- Project Controls Templates
- END OF ADDENDUM NO. 1



BIDDER QUESTIONS AND RESPONSES

- 1. Is a CAD file available for the site? No CAD files are available.
- 2. Is there an engineer's estimate or budget that can be shared with the bidders? No engineers estimate is available.
- **3.** Is the FAA facility and associated duct banks existing, part of this contract, or future installation by others?

The Proposed Electric Duct Bank (By Others) and Communication Duct Banks (By Others) shown on plan sheets 5 and 6 and the Proposed RTR (By Others) shown on plan sheet 5 will be concurrent work installed by others. See Notes 1 and 2 on plan sheet 5 and notes 2 and 3 on plan sheet 6. The Future ASR site shown on plan sheet 6 will be future work installed by others after the completion of this contract.

- Will design loading criteria for the aircraft display be given, or is design loading a contractor design responsibility as well?
 Addendum 1 removes the foundation design from the Contract. A line item for the foundation has been added.
- Please provide the geotechnical report for the project area.
 The geotechnical report is provided for information only as part of Addendum 1.
- **6.** During the prebid meeting it was mentioned that rough grading at the site was complete and that all we are responsible for in this contract is the fine grading. However, we see 32,500 CY of unclassified excavation as part of the bid items in this contract leading us to believe additional rough grading is necessary. Could additional information be provided as to what was meant in the prebid meeting when it was said that all rough grading would be completed ahead of time by others?

During the pre-bid meeting, it was noted the rough grading was substantially complete by others. The Overlook contractor will be responsible for removal of topsoil and cutting in all ditches. There may be other minor rough grading required by the contractor to complete the project. It was also discussed during the meeting that the controlled fill placed by others will require monitoring for settlement by the Overlook contractor and that S&ME will be on-site and provide direction when various activities could occur based on settlement plate readings installed by the Overlook contractor.

- Is it possible for an installation duration to be provided for the playground equipment and canopy accessories? The installation duration for the playground equipment is estimated to be approximately 6 weeks.
- 8. Can an allowance be established for water and sewer tap fess / impact fees for the project? The city of Charlotte does not have a fixed rate schedule for this. These fees have been paid by CLT Airport. The only related water fees the Contractor is responsible for are the 3rd party testing of the backflows.
- **9.** Please confirm that concrete light pole bases are to be provided by Duke and not the contractor.

The lighting provided and installed by Duke Energy do not require concrete light pole



bases.

10. Can you confirm that only NC DOT DBE firms are counted towards the project goal and not NC DOT SBE firms or CBI firms.

Confirmed. While the Prime may utilize a combination of DBE and Non-DBE firms, only NCDOT DBE firms can be counted for participation towards the 17.00% DBE project goal.

- Can a specification be provided for plumbing fixtures to be installed in the restrooms? Sheet P-1, Plumbing Fixture Schedule is a list of fixtures and descriptions of each fixture. See Specification Commercial Water Closet Section 224213.13, Commercial Urinals Section 224213.16 C, Commercial Sinks Section 224216.16, Wash Fountains Section 224233, Drinking Fountains Section 224713.
- **12.** Restroom building elevations on sheet A2 indicated 3" insulation (R-13) beneath EIFS finish. The specifications call for R-15 insulation which would be 4". Please confirm what thickness EIFS to provide.

See Sheet A-3 Details 1 and 2 above top of CMU reads, "EIFS on 3" Insulation (R-15) on 5/8" Ext. Gyp Bd. on 2 x 4 Wd. Studs at 16" o.c." The total thickness of the EIFS is 4".

- The invitation to bid indicates the use of Primavera P6 scheduling software version 18 or higher. Would the use of Phoenix project scheduling be an acceptable alternate? Primavera P6 is required.
- **14.** Can the current version of the Davis Bacon wage rates be provided for this project? Information will be provided in Addendum 2.
- **15.** Do we need to input the unit prices into e-builder in addition to our bid form? Is it possible to remove the need for entering them on e-builder due to the high number of unit prices to be entered on multiple forms.

In addition to the Bid Forms and Supplements within the Invitation to Bid, responsive bidders must enter unit costs into the Response Form in the e-Builder portal. Bidders may use "Download Bid Cost" to access an Excel version of the form and "Upload Bid Cost" to import their populated pricing.

16. We attempted to view the site late last week but the site was gated by ES Wagner and we could not access the site. Could a formal site visit be scheduled to allow bidders to access the actual site?

A site visit will not be provided for this project. The site is currently located in a restricted access area and under construction thru the bidding of the Overlook. Bidders are advised not to attempt accessing these areas for safety and security reasons.

PROJECT MANUAL (FRONT END) REVISIONS

• Remove and replace pages BID-22 through BID-28 from the Project Manual (Front End) with the attached revision.



TECHNICAL SPECIFICATION REVISIONS

- Remove and replace the Table of Contents found on page 5 of 636 in the Technical Specifications with the attached revision.
- Remove and replace the **SP-Chain Link Fence** content found on page 26 of 636 in the Technical Specifications with the attached revision.
- Remove and replace the **SP-Aircraft Display (Allowance No. 1)** content found on page 29 of 636 in the Technical Specifications with the attached revision
- Add the attached Special Provision SP-Chain and Bollard Barrier to the Technical Specifications
- Add the attached Special Provision **SP-Aircraft Display Foundation** to the Technical Specifications

LINE ITEM REVISIONS

NOTE: Bid submissions must include the revised Bid Form and Supplements, attached within this Addendum No. 1.

The following Line Items have been modified to read as follows:

Line Item	Spec No.	Description	Qty	Unit	Unit Price	Amount
31	848	4" CONCRETE SIDEWALK	4900	SY		
32	SP	FENCE, 48" CHAIN LINK	0	LF		
35	SP	12' 48"- CHAIN LINK DOUBLE SWING GATE (Aircraft Display)	0	EA		
NEW	SP	AIRCRAFT DISPLAY CONCRETE FOUNDATION	1	LS		
NEW	SP	BOLLARD AND CHAIN BARRIER	26	EA		

PLAN SHEET REVISIONS

- **Delete** plan sheet Title Sheet and **Insert** the revised plan sheet Title Sheet attached to this Addendum.
- **Delete** plan sheet 3A-9 and **Insert** the revised plan sheet 3A-9 attached to this Addendum.



- **Delete** plan sheet 3B-1 and **Insert** the revised plan sheet 3B-1 attached to this Addendum.
- **Delete** plan sheet 3B-2 and **Insert** the revised plan sheet 3B-2 attached to this Addendum.
- **Delete** plan sheet 3B-3 and **Insert** the revised plan sheet 3B-3 attached to this Addendum.
- **Delete** plan sheet 5 and **Insert** the revised plan sheet 5 attached to this Addendum.
- **Delete** plan sheet PM-3 and **Insert** the revised plan sheet PM-3 attached to this Addendum.
- **Delete** plan sheet LS-1 and **Insert** the revised plan sheet LS-1 attached to this Addendum.
- **Delete** plan sheet LS-3 and **Insert** the revised plan sheet LS-3 attached to this Addendum.
- **Delete** plan sheet SIGN-3 and **Insert** the revised plan sheet SIGN-3 attached to this Addendum.
- **Delete** plan sheet LE-1 and **Insert** the revised plan sheet LE-1 attached to this Addendum.
- **Delete** plan sheet E-4 and **Insert** the revised plan sheet E-4 attached to this Addendum.
- **Delete** plan sheet ITS-4 and **Insert** the revised plan sheet ITS-4 attached to this Addendum.
- **<u>Add</u>** the attached Plan Sheet 3A-10 to the Plans.
- Add the attached Plan Sheet S-2 to the Plans.

END OF ADDENDUM NO. 1



Geotechnical Engineering Report CLT NEAT – Temporary Airport Overlook Drive Charlotte, North Carolina S&ME Project No. 1335-19-012

PREPARED FOR

STV Engineers, Inc. 900 W. Trade Street, Suite 715 Charlotte, North Carolina 28202

PREPARED BY

S&ME, Inc. 9751 Southern Pine Boulevard Charlotte, North Carolina 28273

March 29, 2021



March 29, 2021

STV Engineers, Inc. 900 W. Trade Street, Suite 715 Charlotte, North Carolina 28202

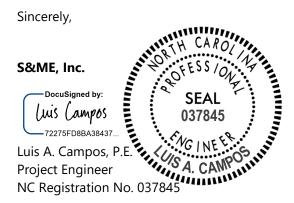
Attention: Mr. John Johnson, P.E.

Reference: Geotechnical Engineering Report CLT NEAT – Temporary Airport Overlook Drive Charlotte, North Carolina S&ME Project No. 1335-19-012 NC PE Firm License No. F-0176

Dear Mr. Johnson:

S&ME, Inc. has completed the subsurface exploration for the Temporary Airport Overlook Drive portion of the Charlotte Douglas International Airport's North End-Around Taxiway project. This study was performed in general accordance with our proposal No. 34-1800303 dated August 31, 2018. The purpose of this study was to determine the subsurface conditions along the proposed roadway alignment so that those conditions can be evaluated regarding the appropriate foundation and construction considerations for the relocated roadway. This report presents the findings of our investigation and pavement recommendations.

S&ME appreciates the opportunity to assist you during this phase of the project. If you should have any questions concerning this report or if we may be of further assistance, please contact us.



Kristen H. Hill, Þ.E., P.G Principal Engineer



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Test Location Plan, Figure 1 Hand Auger Boring Logs (3 sheets) Kessler DCP Logs (2 sheets)



1.0 Project Overview

1.1.1 Project Description

Project information is based on e-mail correspondence between Nikki Honeycutt of STV and Luis Campos of S&ME in January 2021.

As part of the Charlotte Douglas International Airport North End-Around Taxiway (NEAT) project, STV is providing design services to relocate a portion of Airport Overlook Drive to assist with construction sequencing. For the purposes of this report, this portion of the project consists of alignments -L- (Old Dowd Road) and -Y- (Proposed Airport Overlook Drive). Our services were requested to provide roadway and pavement thickness recommendations for the relocated portion of Airport Overlook Drive.

STV provided plan, profile, and cross-section information for both -L- and -Y-. Based on the provided information, no roadway/pavement improvements are planned along -L- except for tie in points to -Y-. The new -Y- alignment typical section is a two-lane roadway. The alignment will require up to 6 feet of cut and 7 feet of fill to achieve design grades. The following traffic information was provided by STV for the -L- alignment, which we understand is the basis of design for the -Y- alignment.

Design Information	Old Dowd Road (-L-)
ADT year 2021	3,700
ADT year 2040	6,500
Tractor Trailer Semi Truck (TTST) %	1
Duals (%)	1
Design Hourly Volume (DHV)	10
Directional Split, %	55

Table 1-1: Traffic Loading Summary

1.1.2 Site Description & Geology

The project site is located in Charlotte, Mecklenburg County, North Carolina. The new Airport Overlook Drive alignment (-Y-) traverses a partially wooded area beginning on the south side of Old Dowd Road (-L-) for about 500 feet, at which point it connects into the existing Airport Overlook Drive over a span of about 180 feet. The pavement surface on the existing Airport Overlook Drive is asphalt.

The project site is located in the Charlotte Belt of the Piedmont Physiographic Province of North Carolina. The Piedmont Province generally consists of well-rounded hills and ridges, which are dissected by a well-developed system of draws and streams. The Piedmont Province is predominantly underlain by metamorphic rock (formed by heat, pressure and/or chemical action) and igneous rock (formed directly from molten material), which were initially formed during the Precambrian and Paleozoic eras. The volcanic and sedimentary rocks deposited in the Piedmont Province were the host for the metamorphism and were changed to gneiss and schist. The more recent

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Paleozoic era had periods of igneous emplacement, with at least several episodes of regional metamorphism resulting in the majority of the rock types seen today.

The topography and relief of the Piedmont Province have developed from differential weathering of the igneous and metamorphic rock. Because of the continued chemical and physical weathering, the rocks in the Piedmont Province are now generally covered with a mantle of soil that has weathered in place from the parent bedrock. These soils have variable thicknesses and are referred to as residuum or residual soils. The residuum is typically finer grained and has higher clay content near the surface because of the advanced weathering. Similarly, the soils typically become coarser grained with increasing depth because of decreased weathering. As the degree of weathering decreases, the residual soils generally retain the overall appearance, texture, gradation and foliations of the parent rock. Alluvial soils, consisting of interbedded sands, silts, and clays, are common in the floodplain along rivers and creeks in the Piedmont.

The boundary between soil and rock in the Piedmont is not sharply defined. A transitional zone termed "Weathered Rock" or "Partially Weathered Rock" is normally found overlying the parent bedrock. Weathered Rock (WR) is defined for engineering purposes as residual material with Standard Penetration Resistances (N-values) exceeding 100 blows per foot. The transition between hard/dense residual soils and WR occurs at irregular depths due to variations in degree of weathering. A depiction of typical weathering profiles in the Piedmont Province is presented in Figure 1-1.

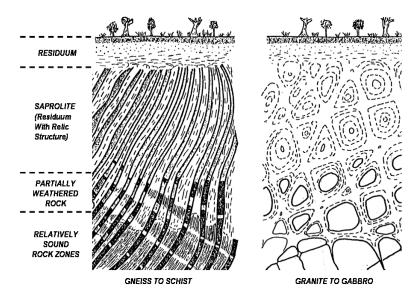


Figure 1-1: Typical Piedmont Weathering Profiles

Water is typically present in the residual soils and within fractures in the PWR or underlying bedrock in the Piedmont. On upland ridges in the Piedmont, water may or may not be present in the residual soils above the PWR and bedrock. Fluctuations in water levels are typical in residual soils and PWR in the Piedmont, depending on variations in precipitation, evaporation, and surface water runoff. Seasonal high water levels are expected to occur during or just after the typically wetter months of the year (November through April).



2.0 Exploration Procedures

2.1 Field Testing

In order to explore the general subsurface conditions at the site, S&ME performed hand auger borings, pavement cores, and Kessler Dynamic Cone Penetrometer (DCP) tests. The field tests were advanced at the 5 locations approximately shown on the Site Plan (Figure 1) in the Appendix between February 16 and March 17, 2021. The test locations were selected by S&ME. Each of the test locations were located in the field by S&ME personnel by measuring distances and estimating right angles from existing site features or using a hand-held GPS unit. Northings and eastings presented on the logs are approximate. Also, elevations presented on our logs should also be considered approximate as these were interpolated from the provided topographical information.

2.1.1 Hand Auger Borings

Hand auger borings HA-1050, HA-1250, and HA-1450 were performed to depths of 6.5 to 10 feet below the existing ground surface. Dynamic Cone Penetrometer (DCP) tests were performed in the hand auger borings at approximate 2-foot intervals. DCP tests were performed in general accordance with ASTM Specialty Publication STP 399. The results of the classifications as well as the DCP tests results are presented on the Hand Auger Boring Logs included in the Appendix.

The DCP test procedure is as follows: The cone point of the penetrometer is first seated 2 inches into the bearing materials to assure that the point is completely embedded. Then the cone point is driven an additional 1-3/4 inches using a 15-pound weight falling 20 inches. The penetrometer reading is the number of blows required to drive the cone point 1-3/4 inches. The cone point may be driven a second and third increment of 1-3/4 inches each and the penetrometer readings are recorded. The penetrometer reading is similar to the Standard Penetration Resistance "N-value" as defined by ASTM D1586. When properly evaluated, the penetrometer test results provide an index for estimating soil strength and relative density. The following figure (from ASTM Special Technical Publication #399, 1966) presents generally accepted correlations between average DCP blow counts and the SPT N-value.

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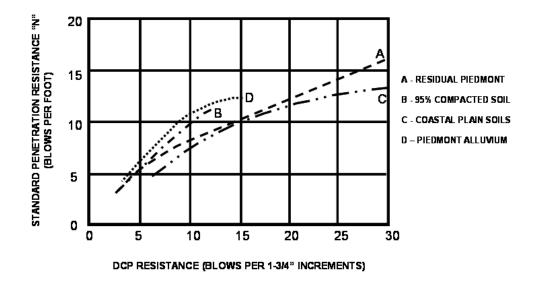


Figure 2-1: Correlation between DCP Blow Counts and SPT N-Values

Groundwater level measurements were attempted in the hand auger borings at termination. Upon completion of the groundwater level measurements, the boreholes were backfilled with soil cuttings.

2.1.2 Pavement Coring and Kessler DCP Testing

The existing pavement was cored at test locations C-1500 and C-1650 using a 4-inch diameter core drill. The pavement thickness was measured and recorded along with the thickness of the stone underlying the asphalt. The core locations were then repaired with cold-mix asphalt patch.

The subgrade soils beneath the existing pavements were then evaluated by performing Kessler DCP tests (ASTM D6951) at both core locations. The Kessler DCP was driven into the subgrade soils by dropping a Dual-Mass Hammer (17.6-pound) from a height of 22.6 inches. The depth of cone penetration is measured at selected penetration of hammer drop intervals and the soil shear strength is reported in terms of DCP index. The DCP index is used to estimate weighted average field CBR values. The penetration rate of the Kessler DCP can be used to estimate in-situ California Bearing Ratio (CBR) and shear strength of near surface soils. The Kessler DCP Test Results sheets are included in the Appendix.

2.2 Laboratory Testing

A staff professional visually examined each sample in general accordance with the American Association of State Highway and Transportation Officials (AASHTO) to estimate the distribution of grain sizes, plasticity, organic content, moisture condition, color, presence of lenses and seams and apparent geological origin. The results of the classifications, as well as the field test results, are presented on the individual boring logs included in the Appendix. Similar materials were grouped into strata on the logs. The strata contact lines represent approximate boundaries between the soil types; the actual transition between the soil types in the field may be gradual in both the horizontal and vertical directions.

3.0 Subsurface Conditions

3.1 Soil Conditions

Soils encountered during this investigation consisted of topsoil underlain by residual materials. The topsoil thickness was measured at 2 inches. The underlying residual soils generally consisted of soft to stiff silty clay (A-7-5), soft to stiff sandy clay (A-6), medium stiff to stiff sandy silt (A-4), and loose to medium dense silty sand (A-2-4). The soils were noted as wet to dry. DCP resistance values in the residual soils ranged from 3 to greater than 25 blows per 1-³/₄ inch increment. The lower DCP resistance values were in the upper portion of the borehole, generally stiffening/hardening with depths.

Groundwater was not encountered at drilling termination. Please note that groundwater levels tend to fluctuate with seasonal and climatic variations, as well as with some types of construction operations.

3.2 Pavement Conditions

Existing pavement conditions were obtained during our field activities on March 17, 2021. During this study, pavement coring and a subgrade evaluation was performed at two locations (C-1500 and C-1650) along the existing Airport Overlook Drive. The approximate test areas are shown in Figure 1 in the Appendix. The subgrade was evaluated by performing Kessler DCP tests and then extending a hand auger boring to collect soil samples. Table 3-1 summarizes the pavement layers encountered:

Test ID	Alignment	Station	Asphalt Thickness (inches)	Base Thickness (inches)	Base Type	Soil Weighted Average Field CBR
C-1500	Proposed Airport	15+00	6 1⁄4	11	ABC	18
C-1650	Overlook Drive (-Y-)	16+50	6 1⁄2	9	ABC	20

Table 3-1: Summary of Pavement Conditions

The subgrade soils at the core locations generally consisted of residual soils which classified as sandy clay (A-6).

The individual Kessler DCP results are presented in the Appendix.

4.0 Conclusions and Recommendations

4.1 Earthwork

4.1.1 Site Preparation

All topsoil, rootmat, vegetation, trash, debris and other unsuitable materials should be stripped to a minimum of 10 feet outside the pavement area. Based on the borings, we anticipate an average stripping depth of 3 inches to remove the surficial materials. Deeper stripping depths should be anticipated in the wooded areas in order to remove the rootmat and localized stumps.

Any existing underground utilities, structures, or obstructions in the proposed construction areas should be properly excavated, removed, abandoned, or re-routed to facilitate the proposed grading. The resulting excavations should be properly backfilled as described later in this report.

4.1.2 Expansive Soils

Results of the hand auger borings and our visual observations of the hand auger samples recovered indicate moderately plastic silty clay (A-7-5) soils exist at the near-surface of the site. These plastic soils, along with high plasticity clays (A-7-6), are common to the region. Along the -Y- alignment, these soils were encountered in boring HA-1450 (corresponding to station 14+50). Plastic soils can undergo change in volume (shrink/swell) with changes in their moisture content. The presence of the moderate to high plasticity material can adversely affect the performance of the pavement systems. Therefore, the presence of these materials should be considered during for design and budgeting purposes.

In order to reduce the risk of damage of the pavement systems, high plasticity (A-7-6) materials, if encountered, should be completely undercut from pavement areas or adequate separation be provided. High plasticity clay (A-7-6) residual soils may remain in place provided they are stable under proofrolling and are separated from design pavement subgrades by a minimum of 2 feet. Separation material should consist of newly placed structural fill soils. Moderately plastic A-7-5 residual soils, such as those encountered in the HA-1450 area, may remain in place provided they are stable under proofrolling; however, we do not anticipate that these materials will be stable as they were soft, as further discussed in Section 4.1.3. Unstable plastic soils should be undercut and replaced with structural fill.

These materials should be carefully evaluated when encountered at/beneath pavement subgrade. An evaluation by the geotechnical engineer's representative should be performed during construction to help reduce the potential of plastic materials from underlying the pavements. Based on the hand auger borings, existing grades, and anticipated grades, we anticipate undercutting of the plastic soils (A-7-5) encountered in between -Y- station 13+50 to the southern tie-in point.

4.1.3 Soft Soils

Relatively soft soils were encountered at the near-surface in two of the three borings performed (HA-1050 and HA-1450).

While we anticipate that the soft near surface soils in boring HA-1050 (corresponding to station 10+50) will be removed due grading (cut) operations, these soft materials should be anticipated at nearby grade points. These materials should be removed and re-worked to achieve adequate compaction.

In the area of boring HA-14-50 (corresponding to station 14+50), these residual soils should undercut due to their plasticity as re-working these materials to re-compact may be difficult. We anticipate that if other soft areas are present, that they may be identifiable during proofrolling activities.

4.1.4 Proofrolling of Subgrade Soils

After stripping of the surficial materials is completed, the exposed subgrade soils in areas to receive fill or at the subgrade elevation in cut areas should be proofrolled with a loaded dump truck or similar pneumatic tired vehicle (minimum loaded weight of 20 tons) to help identify unstable areas requiring surface repair. The proofrolling procedures should consist of four complete passes of the exposed areas, with two of the passes being in a direction perpendicular to the preceding ones. Any areas which deflect, rut or pump excessively during proofrolling or fail to "tighten up" after successive passes should be undercut to suitable soils and replaced with compacted fill.

4.1.5 Subgrade Repair after Exposure

The on-site silts and clays in the project area are fairly low-strength, sensitive to moisture, and can degrade quickly if exposed to water. Because of this, the exposed subgrade soil may deteriorate when exposed to construction activity and environmental changes such as freezing, erosion, softening from ponded rainwater, and rutting from construction traffic.

We recommend that exposed subgrade surfaces in the pavement areas that have deteriorated be properly repaired by scarifying and recompacting immediately prior to additional construction. It should be noted that the level of difficulty and cost of developing a stable subgrade will depend upon the weather conditions before and during construction as well as the time available to stabilize the subgrade. If subgrade preparation operations must be performed during wet weather conditions, undercutting the deteriorated soil and replacing it with compacted crushed stone, rather than soil fill, may be preferable.

We recommend that the grading subcontractor smooth-roll exposed subgrades at the end of each workday, limit construction traffic to defined areas, and protect exposed subgrade soils during construction. This is essential for construction during the typically wetter, cooler months of November through April. If subgrades are rough-graded and not immediately covered by pavement base course materials, the grading subcontractor should cover the exposed subgrades with a sacrificial layer of crushed stone, leave the subgrades approximately 6 inches high, or be prepared to repair/stabilize the subgrades at a later date.

4.1.6 Excavations

Based on the results of the hand auger borings, we anticipate that the majority of the general excavation for this site will be in fill and residual soils. Generally, these soils can be excavated using backhoes, trackhoes, front-end loaders, bull dozers and other types of typical earthmoving equipment. We also estimate that hand auger refusal materials will be excavatable with typical earthmoving equipment.

Weathered rock was not encountered in the hand auger borings. However, weathered rock, intermittent rock lenses, boulders and/or parent bedrock may be encountered during general site grading and excavation for the installation of the roadway and utilities. The depth to, and thickness of, weathered rock and rock lenses or seams, can vary dramatically in short distances and between boring locations; therefore, weathered rock or bedrock may be encountered during construction at locations or depths, between boring locations, not encountered during this exploration.

For temporary excavations, shoring and bracing or flattening (laying back) of the slopes should be performed to obtain a safe working environment. Excavations should be sloped or shored in accordance with local, state and federal regulations, including OSHA (29 CFR Part 1926) excavation trench safety standards. The contractor is usually solely responsible for site safety. This information is provided only as a service and under no circumstances should we be assumed responsible for construction site safety.

4.1.7 Cut and Fill Slopes

Final project slopes should be designed at 3 horizontal to 1 vertical or flatter, which we understand is planned. The tops and bases of all slopes should be located a minimum of 5 feet from pavement limits. The fill slopes should be adequately compacted, as outlined below, and all slopes should be seeded and maintained after construction.

4.1.8 Fill Placement

Structural fill placed within the pavement areas at the site should consist of a low plasticity soil that is free of organic material or debris. Structural fill soils should generally classify as A-1, A-2, A-3, A-4, A-5, or A-6 in accordance with AASHTO. While some of these materials were encountered in cut areas of the site, moderately plastic (A-7-5) materials were also encountered in cut (ditch) areas. These materials can be used as structural fill, however, should not be placed within 2 feet of pavement subgrades. It should be noted that mixing with low plasticity soils may be required to achieve the required compaction criteria.

Structural fill should be placed in 8- to 10-inch-thick loose lifts at moisture contents within three percent of the optimum moisture content of the material as determined by AASHTO T-99 (standard Proctor). Each lift of fill should be uniformly compacted to a dry density of at least 95 percent of the maximum dry density of the material determined according to AASHTO T-99 (standard Proctor), with the upper 8 inches of fill compacted to at least 100 percent.

The geotechnical engineer's representative should perform in-place field density tests to evaluate the compaction of the structural fill and backfill placed at the site. We recommend a testing frequency of one test per lift per

5,000 square feet of fill area in pavements. Also, at least one field density test should be performed for each lift of backfill per every 100 linear feet of utility trench in structural areas.

4.1.9 Groundwater / Dewatering

Groundwater was not encountered at drilling termination, and as such, we do not anticipate that temporary dewatering will be required. However, groundwater may be encountered during construction at depths not indicated by the borings as groundwater levels tend to fluctuate with seasonal and climatic variations, as well as with some types of construction operations.

4.2 **Pavement Recommendations**

4.2.1 Traffic Conditions

Traffic conditions are based on information provided by STV which have been detailed in Table 1-1. As shown in Table 4-1, the following traffic conditions have been incorporated into our analyses:

Alignment	2021 ADT	2040 ADT	Trucks %	Duals %	Design Life (years)	Lanes	Directional Split
(-Y-) Prop. Airport Overlook Drive	3,700	6,500	1	1	20	2	55
(-Y-) Prop. Airport Overlook Drive Overlay	3,700	6,500	1	1	10	2	55

Table 4-1: Traffic Loading Conditions

Considering the NCDOT Pavement Design Procedure (2017), a required total number of 18-kip single axle load applications (ESALs) of **213,067** have been determined for the Proposed Airport Overlook Drive alignment (-Y-), considering a 20-year design life. ESALs of **90,851** have been determined for the Proposed Airport Overlook Drive alignment (-Y-) overlay pavement section, considering a 10-year design life.

4.2.2 Design Recommendations

The asphalt design procedures are based on the NCDOT Pavement Design Procedure (2017) and the 1993 AASHTO Guide for Design of Pavement Structures. The method selected for determining the effective structural number of the existing AC pavement is the Condition Survey for AC Pavements (AASHTO Guide for Design of Pavement Structures). This method is dependent on the types and amounts of deterioration present and involves assigning layer coefficients to the in-place pavements based on the condition.

Generally, the existing pavements along the subject portion of the existing Airport Overlook Drive (-Y-) are in moderate condition as signs of moderate-severity block cracking are present. Based on the condition survey and the condition of the asphalt cores, the top 4 inches of the existing asphalt has been assigned an in-place layer coefficient of 0.30.

Based on the results of the testing program and our engineering judgement regarding soaked CBR values for the soil types encountered, pavement designs have considered a soaked laboratory CBR value of 4.0 along the new portion of the subject -Y- alignment. Also, a reliability of 90%, a standard deviation of 0.45, and a terminal serviceability (Pt) of 2.5 were used for design. The recommendations for the asphalt and subbase layer sections are shown in the following tables.

Table 4-2: Airport Overlook Drive (-Y-) New Pavement Recommendations

Material	Thickness (inches)	Alternative for Full-Depth Thickness (inches)
Asphalt Surface Course (S 9.5B)	1.5	3
Asphalt Intermediate Course (I 19.0C)	3	4
ABC Stone	8	-

Table 4-3: Airport Overlook Drive (-Y-) Overlay Recommendations

Material	Mill and Replace Thickness (inches)
Asphalt Surface Course (S 9.5B)	1

Please note that our analysis indicates that no additional pavement thickness is required for the -Y- overlay area shown in Table 4-3 as the existing section is sufficient for the future traffic loading over the 10-year design period. As such, we recommend that a minimum 1-inch mill and replace be performed, if desired. We also recommend that sections that show cracking be milled and leveled and that any potholes be removed and replaced with full depth asphalt prior to the overlay.

The early placement of the graded aggregate base course will minimize the deterioration of the prepared soil subgrades. However, some loss of graded aggregate due to rutting and surface contamination may occur prior to final asphalt paving. Some infilling and re-grading of the graded aggregate in conjunction with sweeping with a wire broom may be required.

We recommend that special care be given to providing adequate drainage away from pavement areas to reduce infiltration of surface water to the base course and subgrade materials in these areas. If the subgrade soils are allowed to become saturated during the life of the pavement section, there may be a strength reduction of the materials that could result in a reduced life of the pavement section. All water should be routed away from the pavements via ditches to maintain drainage. Pavement areas should be proofrolled prior to placing structural fill and/or base course. Proofrolling procedures are outlined in previous sections of this report.

Longitudinal cracking is a common issue at the joint between new and existing pavement systems. A pavement reinforcement system such as the Glasgrid® System could be utilized directly beneath the overlaid portion of the existing pavement system and the asphalt surface course of the new pavement system to aid in the prevention of longitudinal cracking.

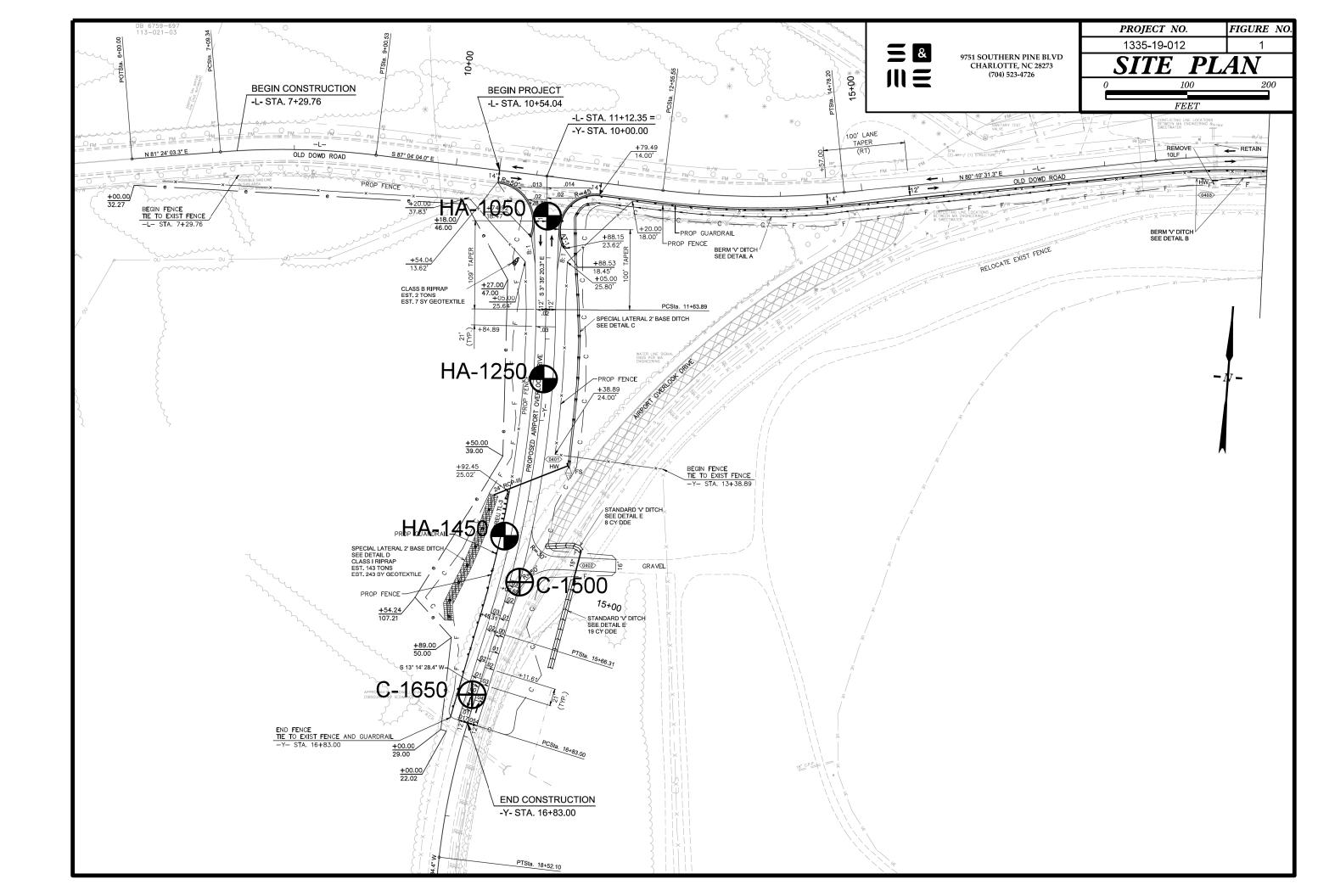


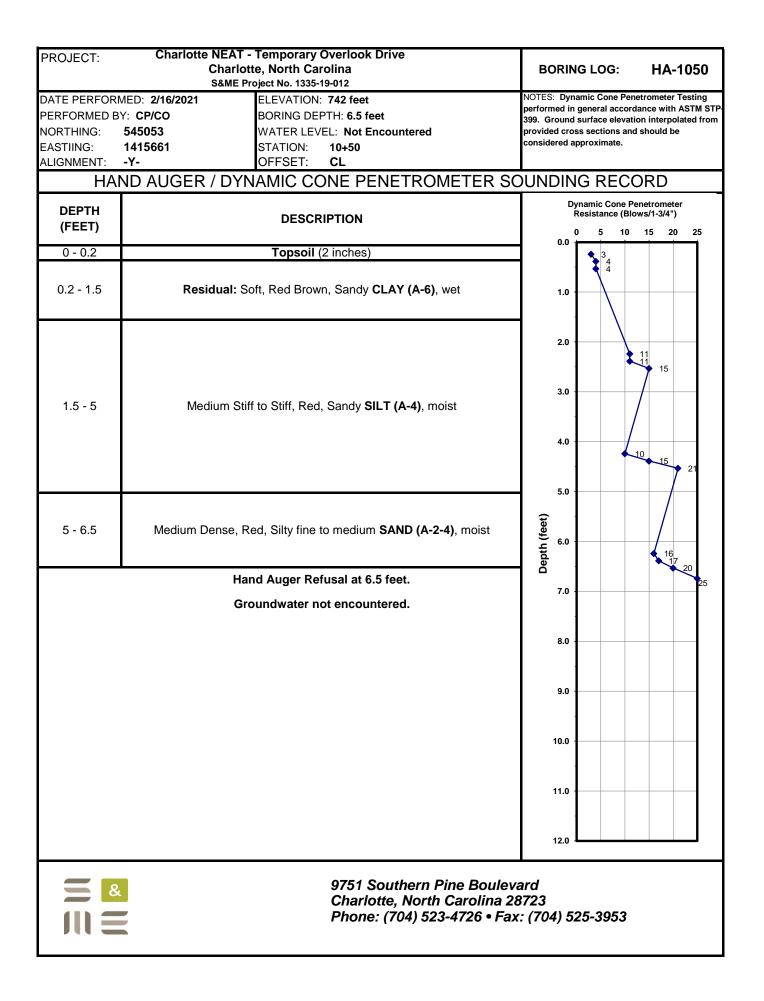
5.0 Qualifications of Report

This report has been prepared in accordance with generally accepted geotechnical engineering practice for specific application to this project. The conclusions contained in this report were based on the applicable standards of our profession at the time this report was prepared. No other warranty, expressed or implied, is made.

The conclusions submitted in this report are based, in part, upon the data obtained from the subsurface exploration. The nature and extent of subsurface variations between the borings may not become evident until construction. If variations appear evident, then the conclusions contained in this report may need to be re-evaluated. In the event that any changes in the nature, design, or location of the structure are planned, the conclusions contained in this report will not be considered valid unless the changes are reviewed by S&ME, and the conclusions of the report are modified or verified in writing.

Appendix





PROJECT:	Charlotte NEAT - Temporary Overlook Drive Charlotte, North Carolina S&ME Project No. 1335-19-012	BORING LOG: HA-1250			
DATE PERFORI PERFORMED B NORTHING: EASTIING: ALIGNMENT: H A	MED: 2/16/2021 ELEVATION: 722 feet	NOTES: Dynamic Cone Penetrometer Testing performed in general accordance with ASTM STF 399. Ground surface elevation interpolated from provided cross sections and should be considered approximate.			
DEPTH (FEET)	DESCRIPTION	Dynamic Cone Penetrometer Resistance (Blows/1-3/4") 0 5 10 15 20 25			
0 - 0.2	Topsoil (2 inches)	0.0			
0.2 - 3	Residual: Medium Stiff to Stiff, Red, Sandy CLAY (A-6) , moist to wet				
3 - 3.5	Medium Stiff, Red, Sandy SILT (A-4), moist	3.0			
3.5 - 5.5	Loose, Red, Silty fine to medium SAND (A-2-4) , moist	4.0 9 ₁₀ 11 5.0			
5.5 - 8.5	Medium Dense, Red Tan, Silty fine SAND (A-2-4) , dry to moist	6.0 7.0 8.0 10 11 17 25			
	Hand Auger Refusal at 8.5 feet.	25			
	Hand Auger Nerusai at 0.3 leet.	9.0			
	Groundwater not encountered.	10.0			
		12.0			
	9751 Southern Pine Boulev Charlotte, North Carolina 28 Phone: (704) 523-4726 • Fax	3723			

PROJECT:		Temporary Overlook Drive				
		te, North Carolina oject No. 1335-19-012	BORI	NG LOG:	HA-1450	
EASTIING: ALIGNMENT:	MED: 2/16/2021 Y: CP/CO 544656 1415638 -Y-	ELEVATION: 707 feet BORING DEPTH: 10 feet WATER LEVEL: Not Encountered STATION: 14+50 OFFSET: 20' RT	performed in 399. Ground provided cro considered a	n general accord d surface elevations sections and approximate.		
HAI	ND AUGER / DYN	IAMIC CONE PENETROMETER SO	JUNDIN	IG RECO	ORD	
DEPTH (FEET)		DESCRIPTION	Dynamic Cone Penetrometer Resistance (Blows/1-3/4") 0 5 10 15 20 25 0.0			
0 - 0.2		Topsoil (2 inches)	0.0	3 ₄		
0.2 - 1.5	Residual: Soft to Me	dium Stiff, Red Brown, Silty CLAY (A-7-5), wet	1.0	5		
1.5 - 3.5	Medium Stiff to S	Stiff, Red Brown, Silty CLAY (A-7-5) , moist	2.0	99	11	
3.5 - 5.5	Medium Stiff to Stiff,	Red Orange, Sandy CLAY (A-6), moist to wet	4.0		12	
5.5 - 7.5	Stiff, Red O	range, Sandy SILT (A-4) , moist to wet	0.6 Depth (feet) 2.0		11 15	
7.5 - 10	Loose to Medium Den	se, Red, Silty fine to coarse SAND (A-2-4) , moist	9.0		15	
	Hand	Auger Terminated at 10 feet.	10.0		12	
		oundwater not encountered.	11.0		24	
			12.0			
	-	9751 Southern Pine Boulev Charlotte, North Carolina 2 Phone: (704) 523-4726 • Fax	8723	25-3953		

KESSLER DCP TEST RESULTS

Project Name: S&ME Project No.: CLT NEAT - Overlook Temporary Roadway 1335-19-012 Ph 01

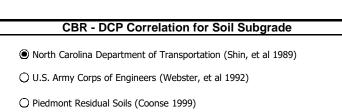
Test Location: Thickness of Stone (in):

C-1500 11 Date: 3/17/2021

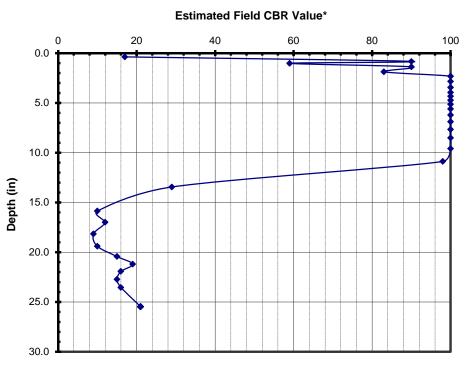
Personnel:

CP

Test Data			
No. of	Cummulative		
Blows	Penetration		
	(mm)		
1	19		
1	23		
1	29		
3	41		
3 5 5 5 5 5 5 5 5 5	54		
3	64		
5	80		
5	95		
5	106		
5	114		
5	127		
	134		
10	151		
10	165		
10	185		
10	204		
10	229		
10	258		
10 8	295 388		
8 1			
1	418 445		
1	445		
1	508		
1	530		
1	547		
1	567		
1	588		
1	608		
5	686		
0	000		



Test Summary						
Stone		Soil Subgrade				
# Values	19	# Values	11			
Average CBR	91	Average CBR	16			
Weighted Avera	92	Weighted Average	18			
Max CBR	100	Max CBR	29			
Min CBR	17	Min CBR	9			



* Stone Field CBR estimated using published NCDOT relationship. Subgrade Field CBR estimated using relationship indicated above.



KESSLER DCP TEST RESULTS

Project Name: S&ME Project No.: CLT NEAT - Overlook Temporary Roadway 1335-19-012 Ph 01

Test Location: Thickness of Stone (in):

C-1650 9

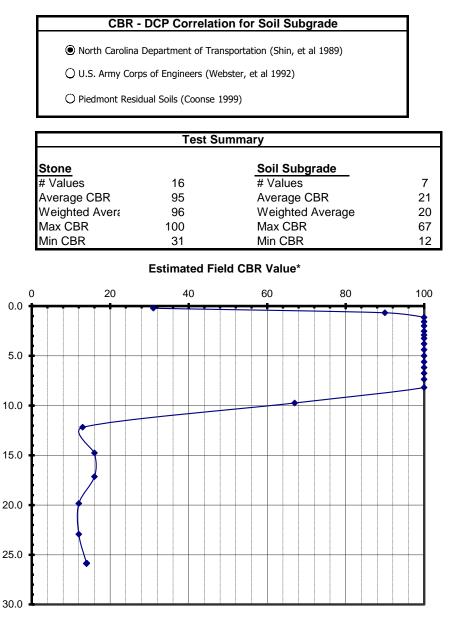
Depth (in)

Date: 3/17/2021

Personnel:

nel: CP

No. of Blows Cummulative Penetration (mm) 1 11 3 23 5 35 5 45 5 56 5 72 5 76 5 89 10 104 10 119 10 135 10 150 10 164 10 179 10 221 10 274 3 345 3 405 3 466 3 543 3 623 3 691	Te	st Data
Blows Penetration (mm) 1 11 3 23 5 35 5 45 5 56 5 72 5 76 5 89 10 104 10 135 10 150 10 164 10 179 10 221 10 274 3 345 3 405 3 543 3 623	No. of	Cummulative
$\begin{array}{c cccc} & & & & & & \\ \hline 1 & & & & 11 \\ \hline 3 & & & & 23 \\ \hline 5 & & & & 35 \\ \hline 5 & & & & & 35 \\ \hline 5 & & & & & 56 \\ \hline 5 & & & & & 72 \\ \hline 5 & & & & & 76 \\ \hline 5 & & & & & 89 \\ \hline 10 & & & & & 104 \\ \hline 10 & & & & & 104 \\ \hline 10 & & & & & 104 \\ \hline 10 & & & & & 104 \\ \hline 10 & & & & & 105 \\ \hline 10 & & & & & 164 \\ \hline 10 & & & & & 179 \\ \hline 10 & & & & & 179 \\ \hline 10 & & & & & 195 \\ \hline 10 & & & & & 221 \\ \hline 10 & & & & & 274 \\ \hline 3 & & & & & 345 \\ \hline 3 & & & & & 405 \\ \hline 3 & & & & & 543 \\ \hline 3 & & & & & 543 \\ \hline 3 & & & & & 623 \\ \end{array}$		Penetration
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3 466 3 543 3 623		
3 543 3 623	3	
3 623	3	
3 691	3	
	3	691



* Stone Field CBR estimated using published NCDOT relationship. Subgrade Field CBR estimated using relationship indicated above.





CHARLOTTE DOUGLAS INTERNATIONAL AIRPORT CHARLOTTE, NORTH CAROLINA

ADDENDUM NO. 1

NORTH END AROUND TAXIWAY (NEAT) OVERLOOK RELOCATION

CLT PROJECT NO. AVIA 23-50 ADDENDUM DATE: MAY 16, 2023

PRE-BID MEETING ATTENDANCE LIST

Name	Organization	Phone Number	Email Address
Alan Johnson	NCDWR		alan.johnson@ncdenr.gov
Andrew "Drew" Vane	STV	(704) 330-9555	andrew.vane@stvinc.com
Cauley Hobson	Chandler Construction		chobson@chandlerconstruction.com
Chris Malinowski	MESSER CONSTRUCTION CO.		
Daren Daye	DMD Supply Services		dmdaye@dmdsupplygroup.com
David Coe	Crowder		dcoe@crowderusa.com
Elyana Vasquez			
Eno Atta			
Eric Rysdon	HDR	(980) 337-5021	eric.rysdon@hdrinc.com
Eve Hibbler	BDC		ehibbler@blythedevelopment.com
Gary Peeples	CLT		ghpeeples@cltairport.com
Gerry McCauley	STV		gerald.mccauley@stvinc.com
Harry Sherrill	STV		hsherrill@stvinc.com
Hunter Ballard	CLT	(980) 287-1377	hunter.ballard@cltairport.com
James Wally	CLT	(980) 307-2335	james.wally@cltairport.com
Jay Trimble	TBE	(704) 426-6070	jtrimble@tbeclt.com
John N. Johnson	STV	(704) 287-9689	john.johnson@stvinc.com
Kevin Andrews	MCCO	(980) 259-0488	kandrews@mcco-us.com
Laura Handleton	КНА	(704) 954-7487	Laura.Handleton@kimley-horn.com
Lindsay Madenford	CLT	(704) 608-4976	lemadenford@cltairport.com
Lisa Markus	Martin Landscaping		MartinLandscaping@rocketmail.com
Logan Childs	Archer Western		lchilds@walshgroup.com
Logan Moran	CLT	(704) 359-4013	logan.moran@cltairport.com
Marcus Choi	CLT		marcus.choi@cltairport.com
Mia Guerrero-Horner	CLT		Mia.Guerrero@cltairport.com
Michael Baril	STV		michael.baril@stvinc.com
Michael Guiliano	STV		michael.guiliano@stvinc.com
Mike Hall			
Mike Schmidt			
Mikkea Carter	CLT		Mikkea.Carter@cltairport.com
Misty			
Renee Stowe	МССО	(704) 301-9462	rstowe@mcco-us.com
Thomas Greiter	HNTB	(312) 237-1390	tgreiter@hntb.com



Name	Organization	Phone Number	Email Address
Tiffani Rogoff	TBE	(704) 426-6070	trogoff@tbeclt.com
Walt Fisher	SHOWALTER CONSTRUCTION		WFisher@showalterconstruction.com
Will Gharst	MESSER CONSTRUCTION CO.		wgharst@messer.com
William Garland	MESSER CONSTRUCTION CO.		
Zach Bornhorst	MESSER CONSTRUCTION CO.		



HOUSEKEEPING ITEMS

- Please keep devices on mute.
- Please hold questions until the end of the meeting. At that time, we will open the floor for questions and answers.
- All questions and answers discussed in this meeting are unofficial and will need to be submitted in writing for an official response.
- This presentation will be included in the first addendum, with an attendance list and bidders list.

AGENDA

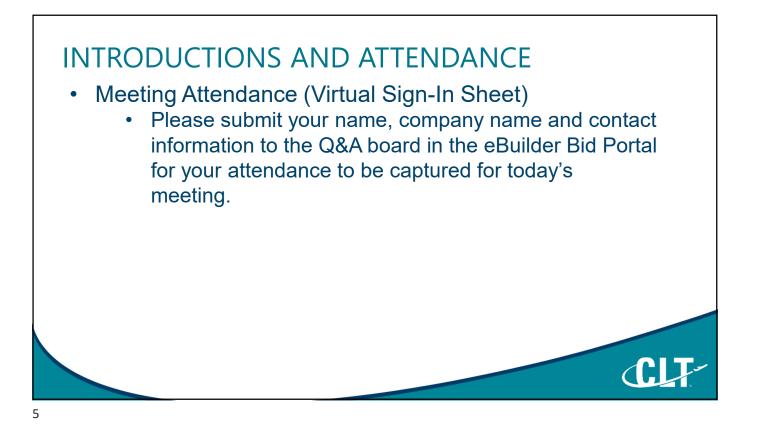
- Introductions and Attendance
- Bid Submission and Opening
- Bid Form and Supplements
- Addenda and Bidder Questions
- eBuilder Bid Portal
- Information for Bidders

- Security
- DBE Program
- Project Scope
- Project Controls
- Questions from Attendees
- Closing

INTRODUCTIONS AND ATTENDANCE

- Presenters:
 - Gary Peeples Contract Compliance Manager (CLT)
 - James Wally Engineering Program Manager (CLT)
 - Jacob Baertlein Engineering Project Manager (CLT)
 - Logan Moran Security Ops Constr. Supervisor (CLT)
 - Mia Guerrero Civil Rights Lead Specialist (CLT)
 - John Johnson, P.E. Engineer of Record (STV)
 - Drew Vane, P.E. Engineer of Record (STV)
 - Harry Sherrill, AIA Architecture Director (STV)
 - Michael Baril, CNNA Engineer of Record (STV)

3





Time: 2:00 PM EST (per CLT's clock)

Location: Accepting Electronic Submissions ONLY through the eBuilder Bid Portal

Public Bid Opening can be viewed via WebEx (Meeting link is in solicitation document)

Anticipated Notice to Proceed: August 2023



ADDENDA AND BIDDER QUESTIONS

- All Addenda will be posted to the eBuilder Bid Portal.
- Questions must be addressed in written form and submitted through the eBuilder Bid Portal Q&A tab.
- Questions and Answers will be included in addenda for the benefit of all bidders. Please submit all questions asked in this pre-bid meeting using the Bid Portal, to be addressed within an Addendum.
- Deadline for submitting written questions will be on Monday, May 22, 2023, at 5:00 PM EST (close of business).

EBUILDER BID PORTAL Charlotte Douglas International Airport (CLT) / Solicitations 010 Development ITB AVIA 23-50 NEAT OVERLOOK RELOCATION					
Status 28 Days 23 Hours 40 Minutes Left Summary 0.00	/				
Bid Package Open Due on 06.01.2023 at 2:00 PM Base Bid Total 0.00 Bidding Pending (GMT-05:00) Eastern Time (US & Canada) Alternate No 1 Total 0.00 Response Submitted No No No No No					
Package Invitation Response Form Q&A Board					
Accept Decline					

INFORMATION FOR BIDDERS Contract Time: ٠ . 227 Calendar Days Minimum Insurance Requirements: Commercial General Liability Insurance and Vehicle Liability Insurance: \$5 Million Each • Liquidated Damages: • Please review Section 2.1 (Liquidated Damages) within the Project Manual Failure to achieve Substantial Completion: \$700.00 per calendar day for each calendar day • the 227 calendar day Contract Time is exceeded. Failure to achieve Final Completion: \$700.00 per calendar day for each calendar day the 33 calendar day Contract Time is exceeded. Intermediate Completion Time (ICT) #1: Septic System Permit Liquidated damages for failure to satisfactorily complete all work associated with the installation of the septic system to obtain the Operation Permit 30 days prior to the Substantial Completion of the Work will be \$500.00 per calendar day, assessed against the Contractor.



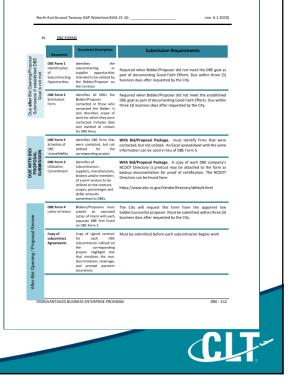
DBE PROGRAM

- DBE Goal: 17.00%
- Goal must be met at time of bid
- Forms 3 and 5 must be filled out in its entirety and signed
- DBE firms can be found on NCDOT's website:

https://www.ebs.nc.gov/VendorDirectory/search.htm I?s=fn&a=new

Questions? Contact Mia Guerrero

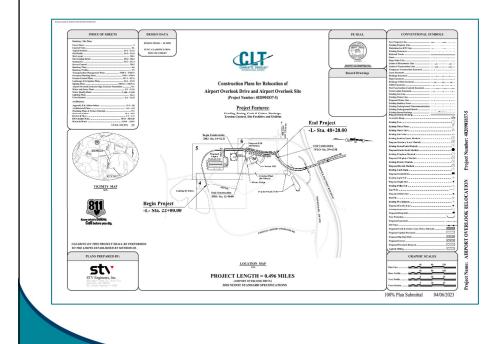
Mia.Guerrero@cltairport.com (980) 433-9874



Copy this Form 3 as needed, to docu v9.10.14 Bidder/Proposer Name: Bidder/Proposer Address:	ment additional subcontracting co	mmitments. In	dicate page # ran	ge: Page of	Charlotte-Douglas		Disadvantaged Business Ente m 5: Schedule of Non- adsheet with the same information	Selected S	ubcontractors	*92
Bidder/Proposer Annual Gross Receipt:	Less than \$500K			Project Name:	Copy this side of Form 5 as needed Project N®					
Bidder/Proposer Age (in years):					Project Nº:				Project N#	
Project Name:					Bidder's Name:			1		,
Project Number:		Esta	blished DBE Go	al:	NAME Of Subcontracting Firm and Annual Gross Receipts (AGR)		Address, Contact Name AND Phone Number	DBE Certified? (Y/N)	Scope of Work	Reason For Not Selec
1. List below all DBEs that you in	tend to use on this contract							(1/14)		
DBE Vendor Name & Address	Description of work / materials	NAICS Code	Reporting Number	Total Projected Utilization (S)	AGR: 4500 5000 10 5000 10 10 - 50 500 -					
Annual Gross Receipt < \$500K 600K-1M 1M-5M >\$5M	Firm Age (in years):				AGR: 4500k					
Annual Gross Receipt: - < \$500K 600K-1M - 1M-5M - >55M	Firm Age (in years):				A GR: 					
Annual Gross Receipt: 🗆 < \$500K 500K-111 🔲 1M-5M 🔲 >55M	Firm Age (in years):									
2. List below all Non-DBEs that y					4500k S00k - IN I IN - SM SM -					
Vendor Name & Address	Description of work / materials	NAICS Code	Reporting Number	Total Projected Utilization (\$)	AGR:					
Annual Gross Receipt: □< \$300K 500K-1M □ 1M-5M □ >\$5M	Firm Age (in years):				☐ 4500 ☐ 500 - 38 ☐ 18 - 58 ☐ 58 - The Undersigned certified that the firm(s) a	bove was (we	e) contacted, in good faith, and tha	t said firm(s) wa	s (were) not selected to partic	ipate in this contract.
Annual Gross Receipt:	Firm Age (in years):				Authorized Signature Printed	Name	Title	Date	_	
A. Total Subcontractor/Supplier Utilizatio B. Total DEE Utilizatio: D. Percent DEE Util.(8+C): Signature 14:00: indicated th a) It has complied with all provision of the D b) Pailver to properly document such complia rejection of bid.	% Must be ro t the undersigned Company certifies and a BE Program:	gree that:	2) decimal places				DBE Foi	rm l	No. 5	

13

PROJECT SCOPE – Work Items Overview



<u>Site</u>

- Grading
- Storm drainage
- Roadway
- Maintenance of Traffic
- Pavement Marking
- Signing
- Sediment/Erosion Control
- Landscape & Irrigation
- Lighting & Electrical
- Water Quality Basin
- · Water and Sewer

Architecture

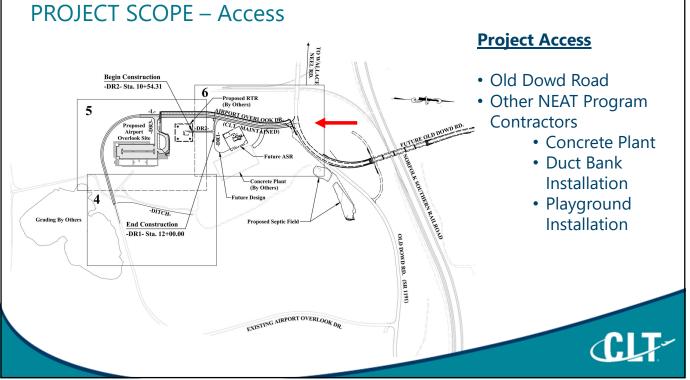
- Toilet Building
- Memorial

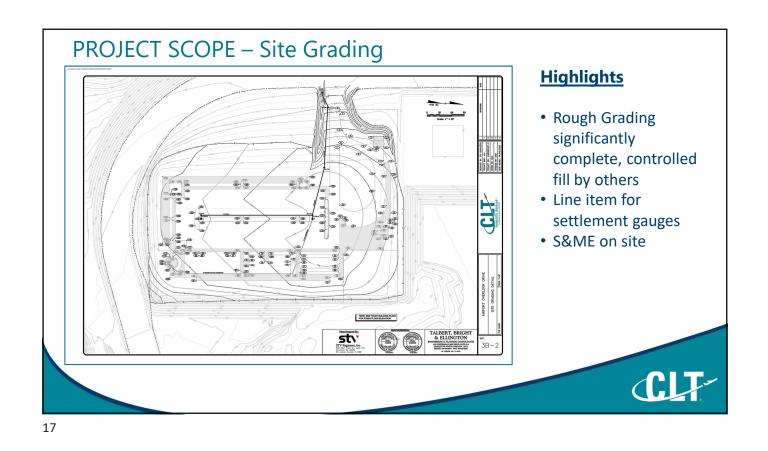
Security

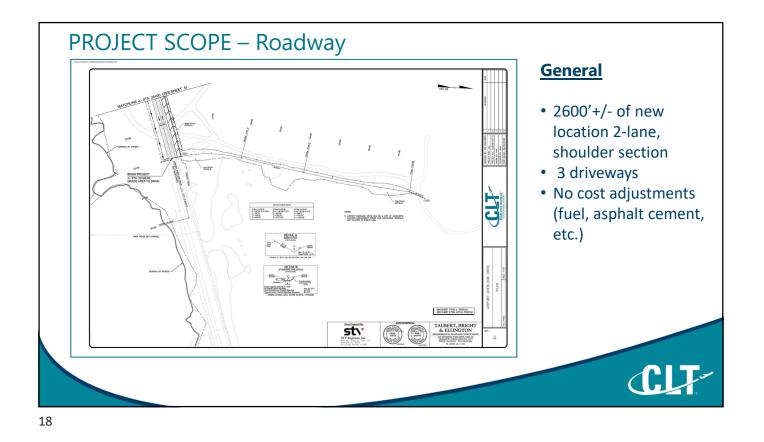
- Cameras
- Blue Light Phone

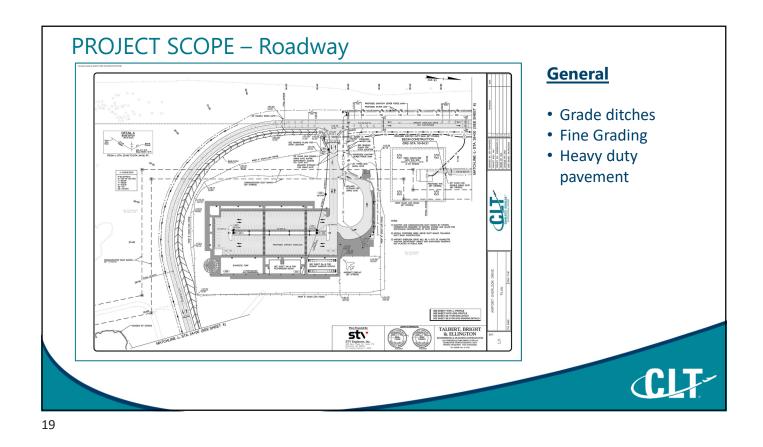
- Wayfinding Signage Airport Overlook Entrance Sign
- Site Signage

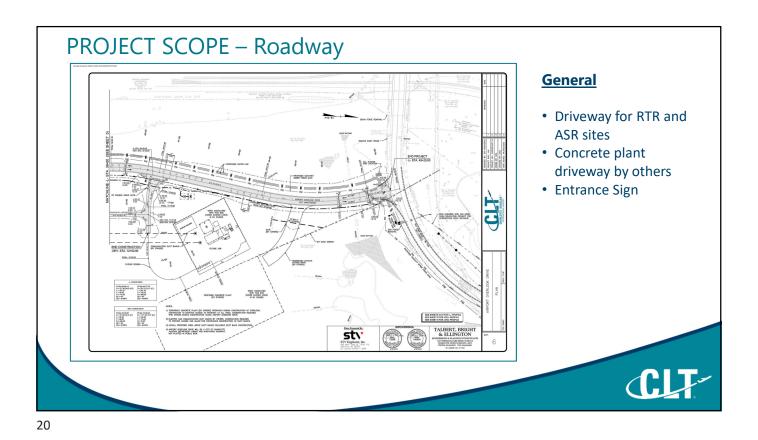


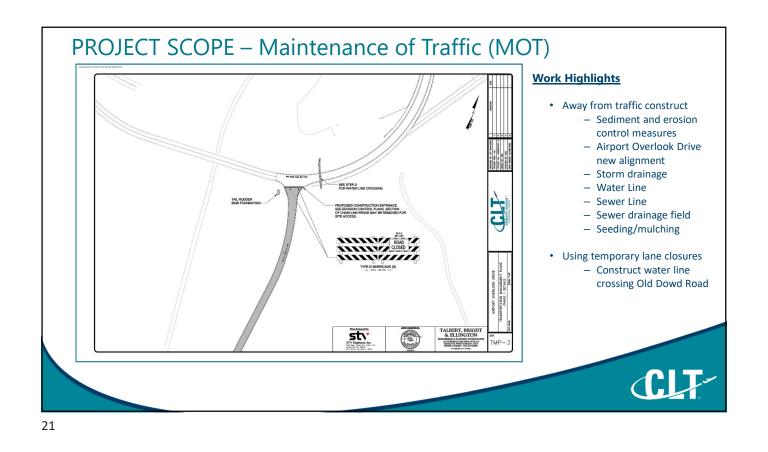


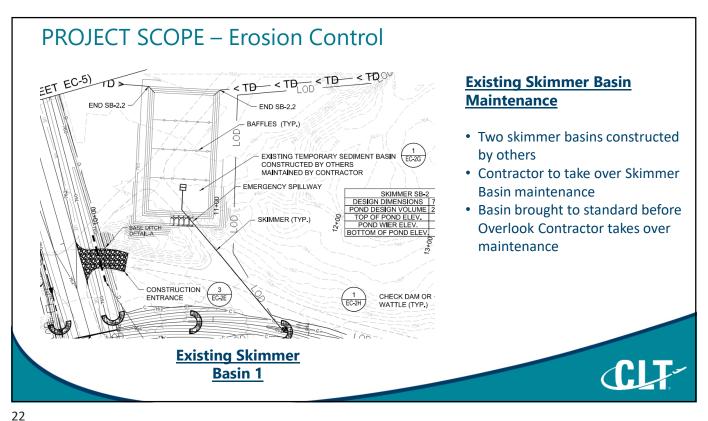


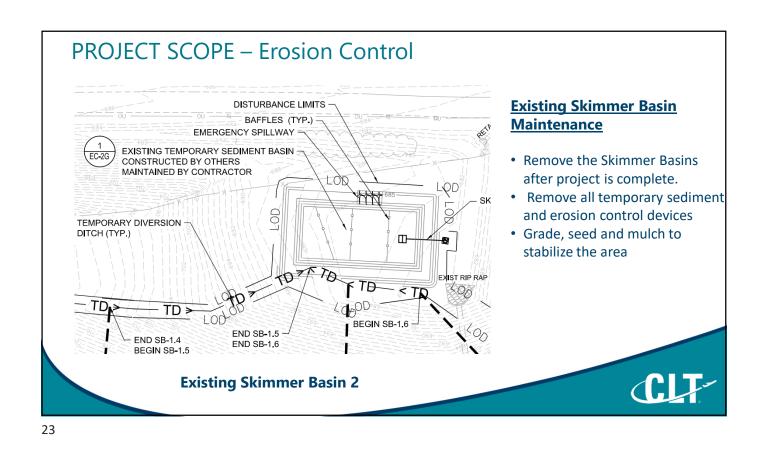


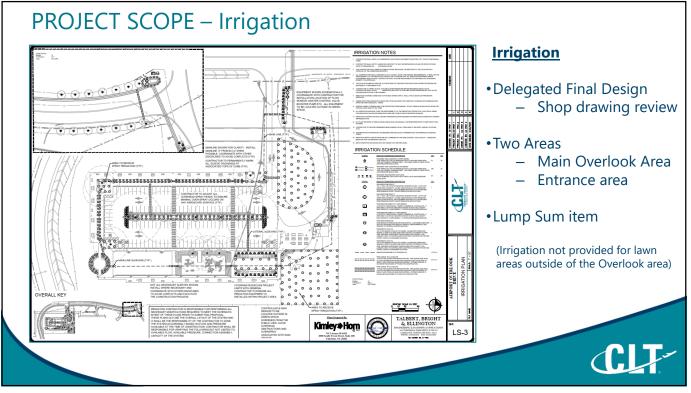


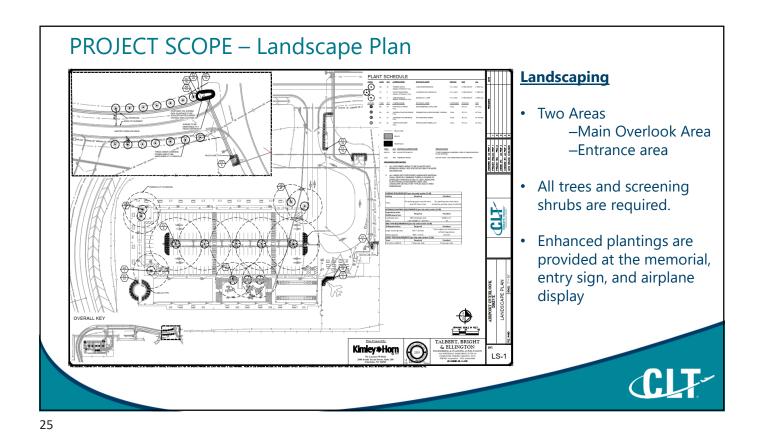


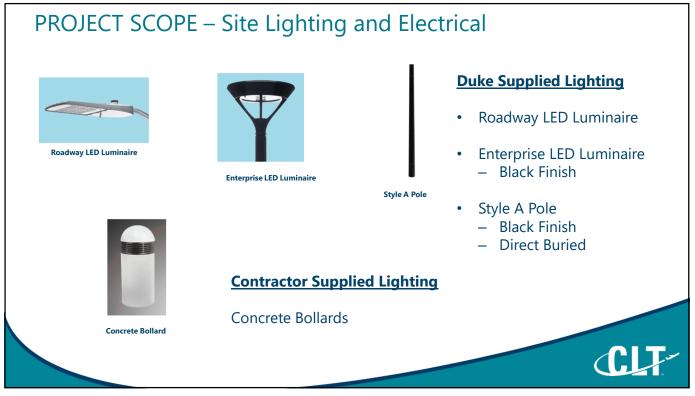


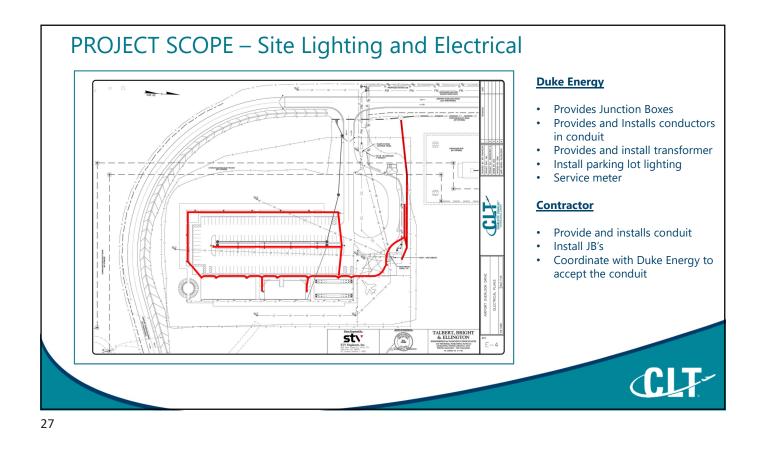


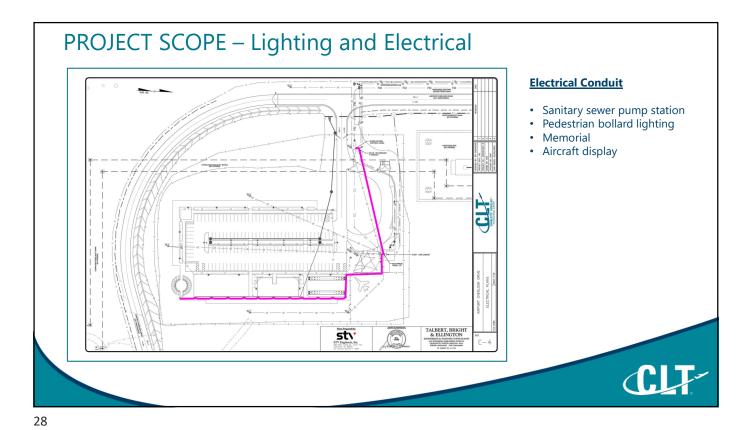


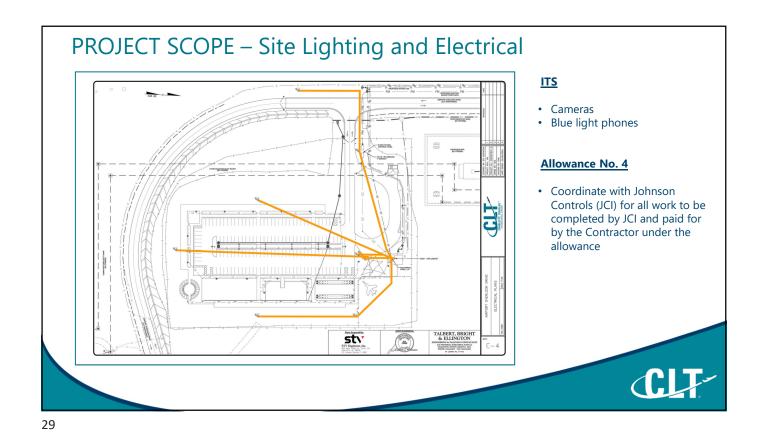


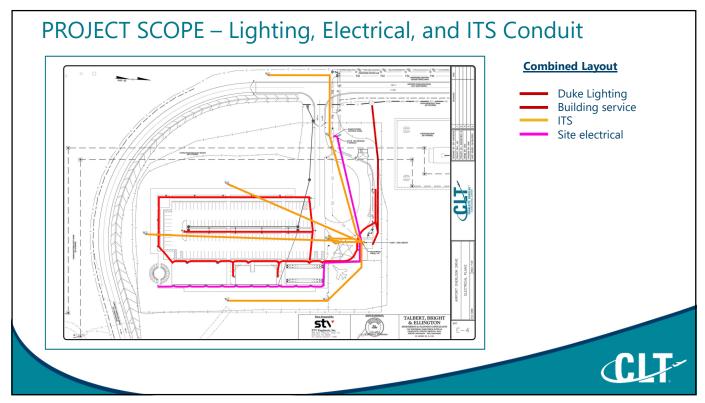


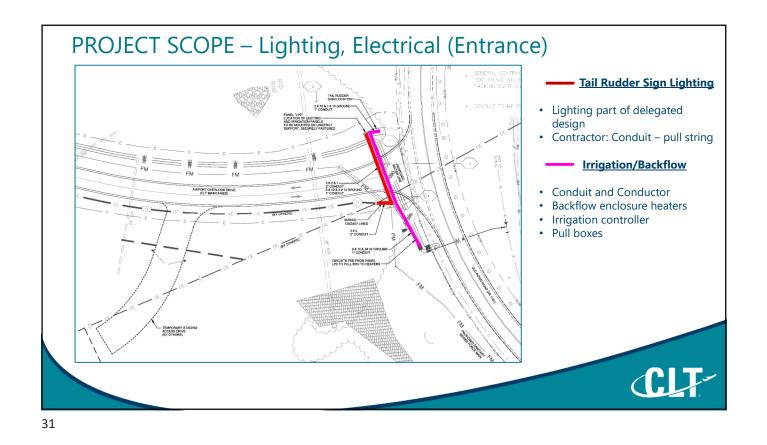


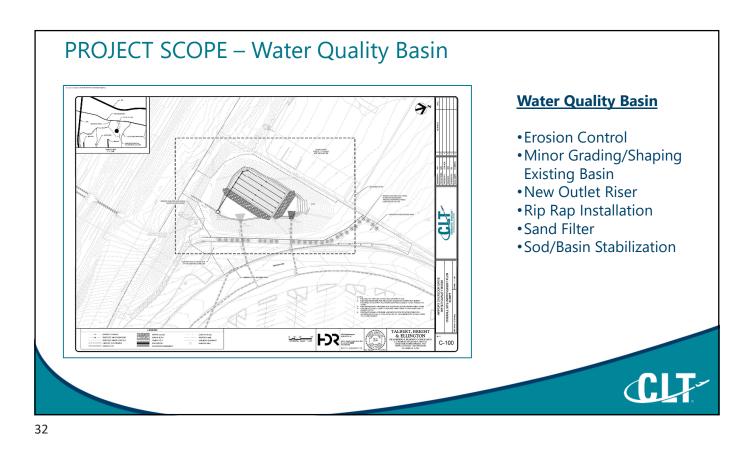






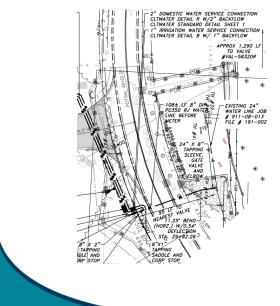




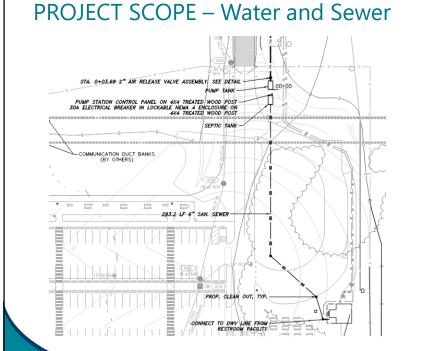


PROJECT SCOPE – Water and Sewer

Water Line Installation



- Connect to existing 24" water main and open-cut across Old Dowd Road.
- Three meters (including one detector meter) & backflows
- Three water lines to restroom facilities in same trench.
 - -1" PVC irrigation
 - -2" PVC domestic
 - -8" DIP fire line
- Private hydrants.
- Will require CLTWater backflow testing/approval via CLTWater approved tester.



Gravity Sewer

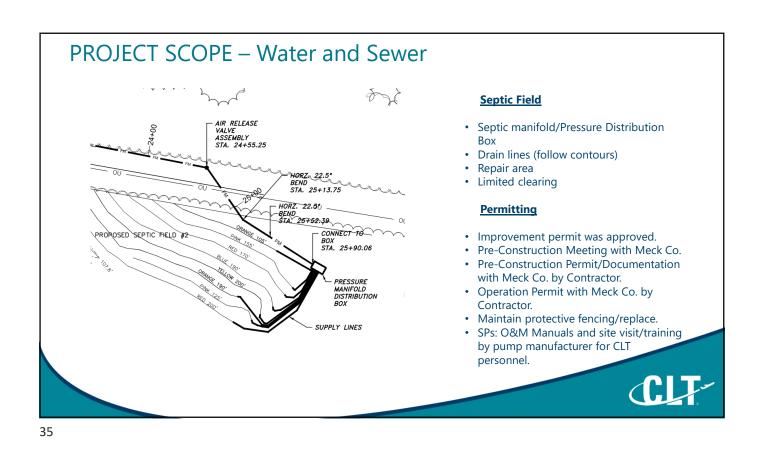
• Approx. 294LF of 6" Gravity sewer to septic tank.

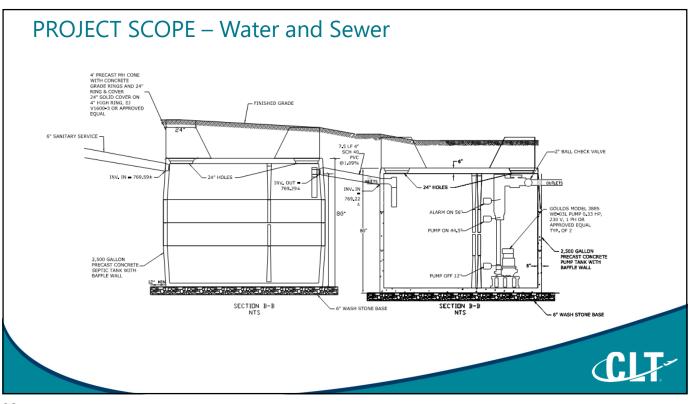
Pumping

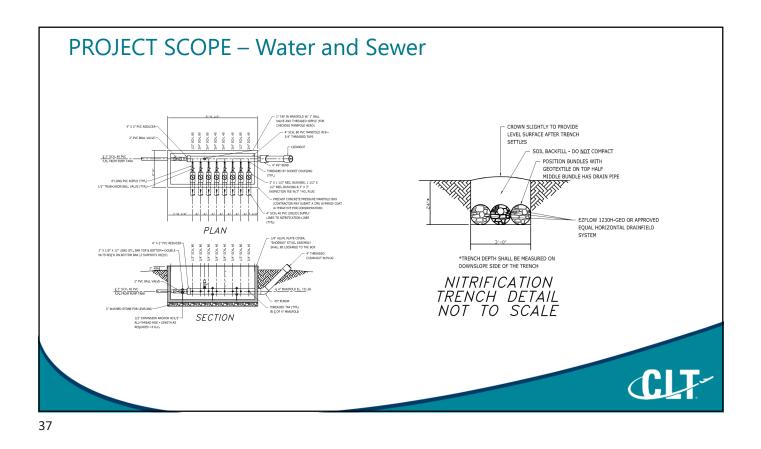
- 2,500 gallon septic tank
- 2,500 gallon pump tank
- Duplex pumps with alarm panel
- 2" PVC Force Main to manifold
- PVC sleeves at some crossings Air release valves

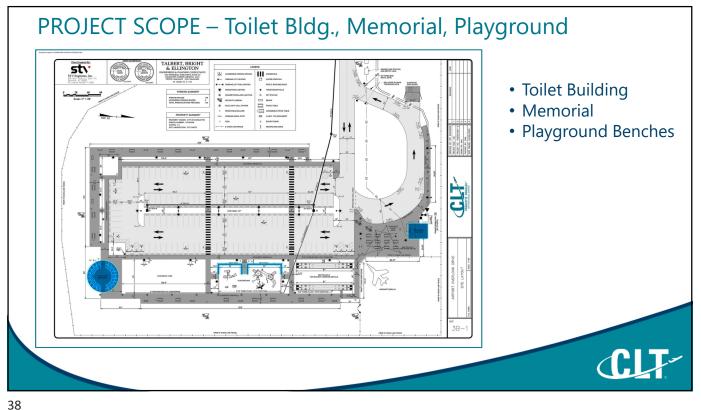
Special Provisions

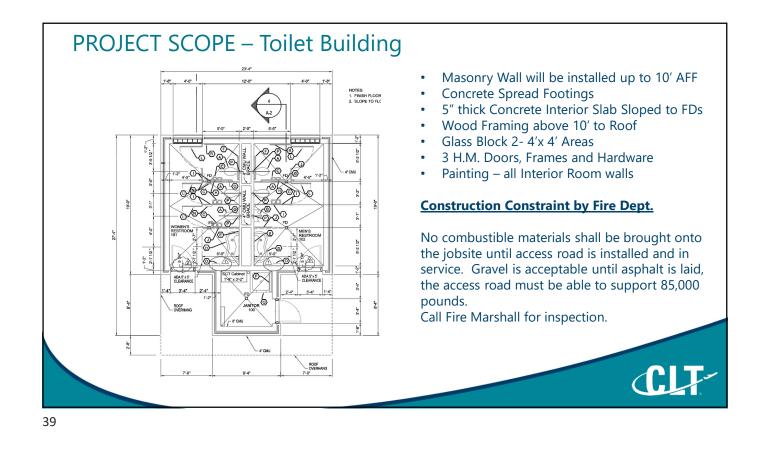
- Licensed septic installer
- Septic installer performs light clearing of septic fields work with Meck County to minimize tree removal.
- Designed around EZ-FLOW system. Contractor may submit other nitrification system, subject to approval by Mecklenburg County GWS.

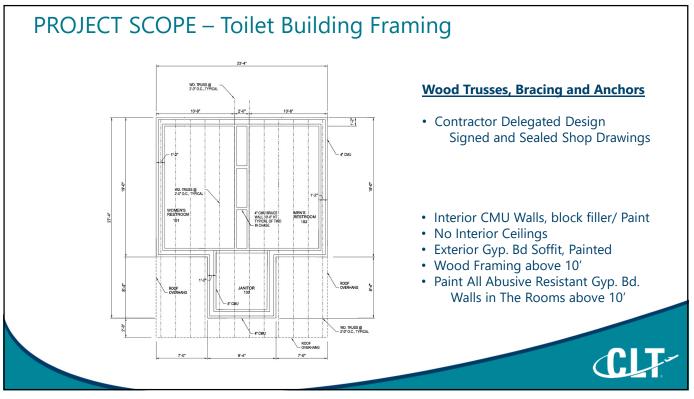


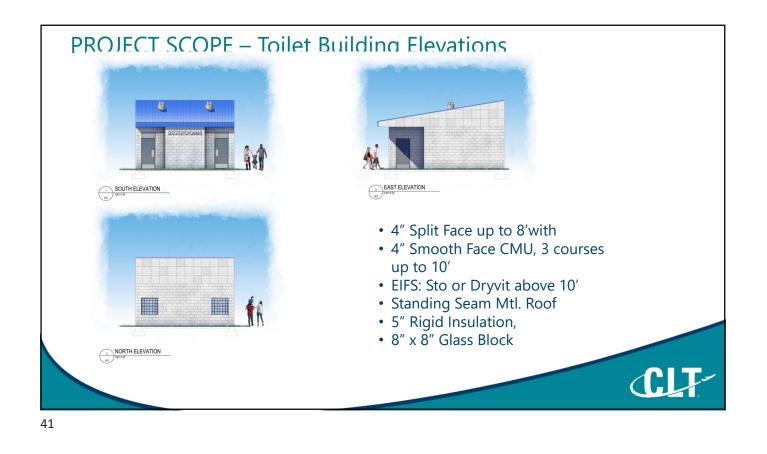


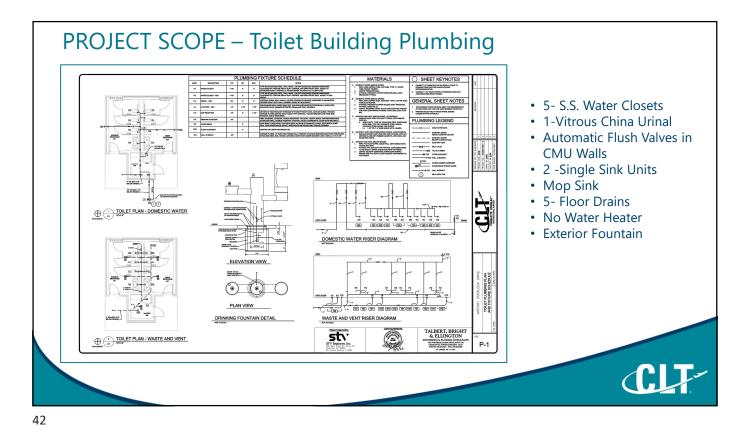


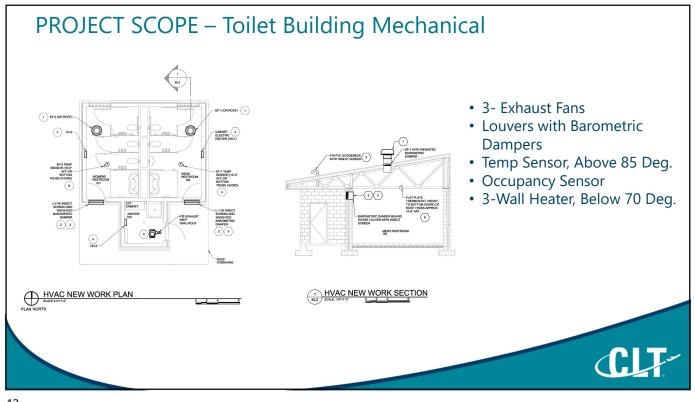


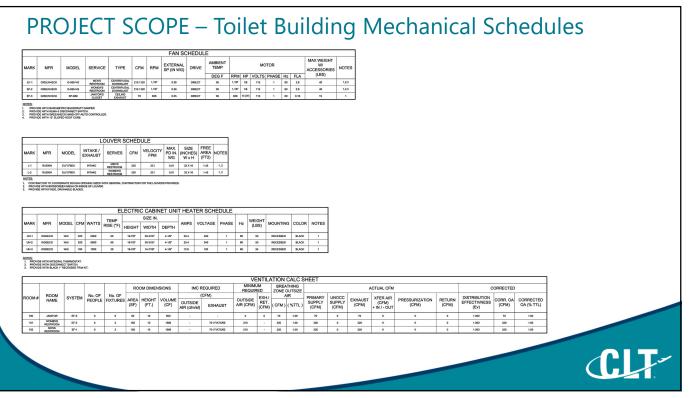


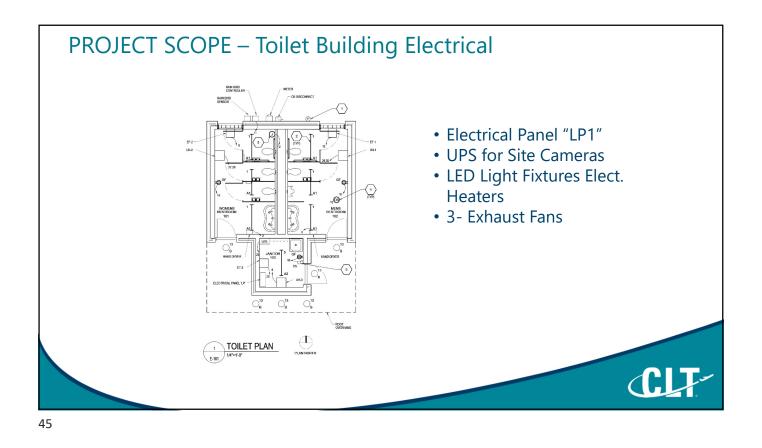




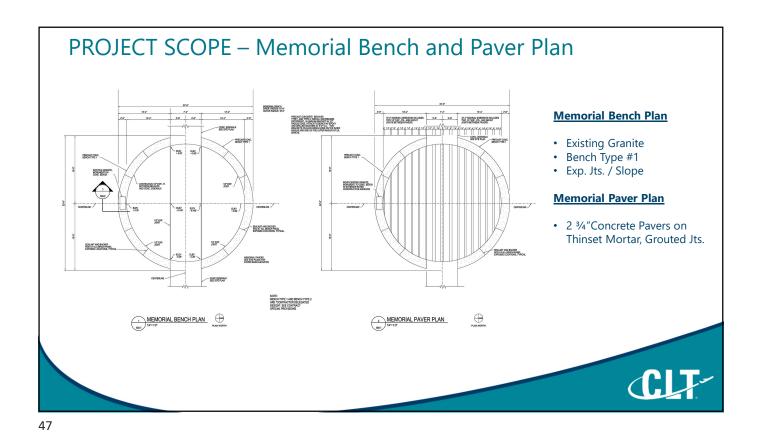


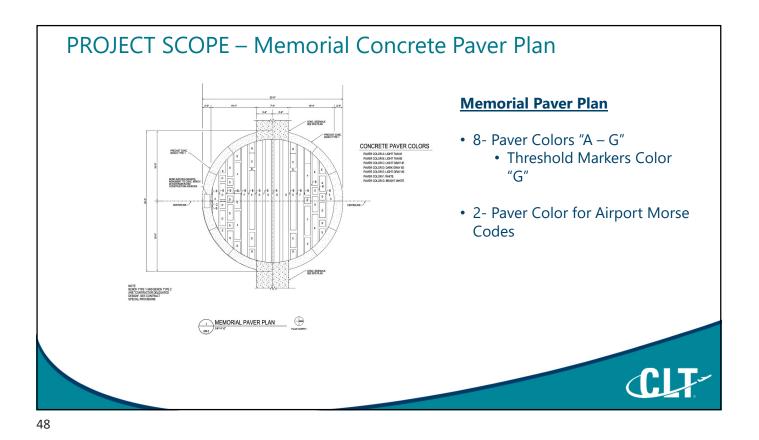


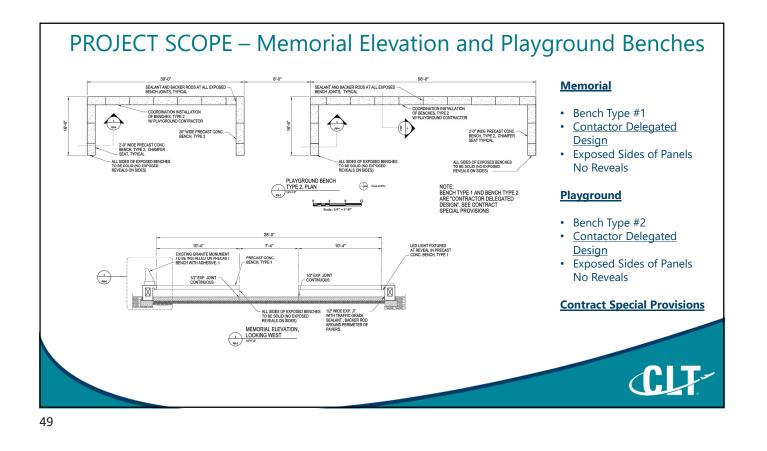


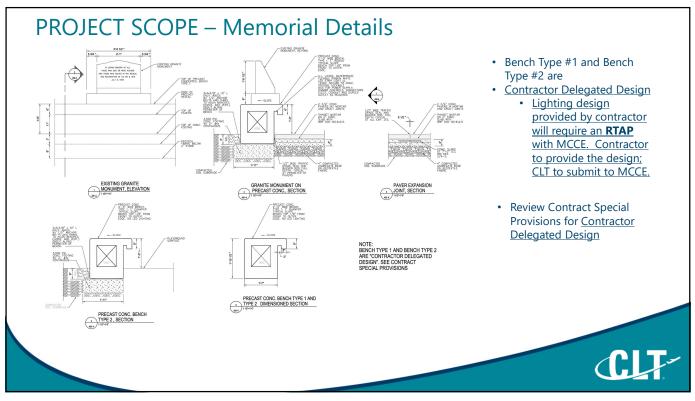


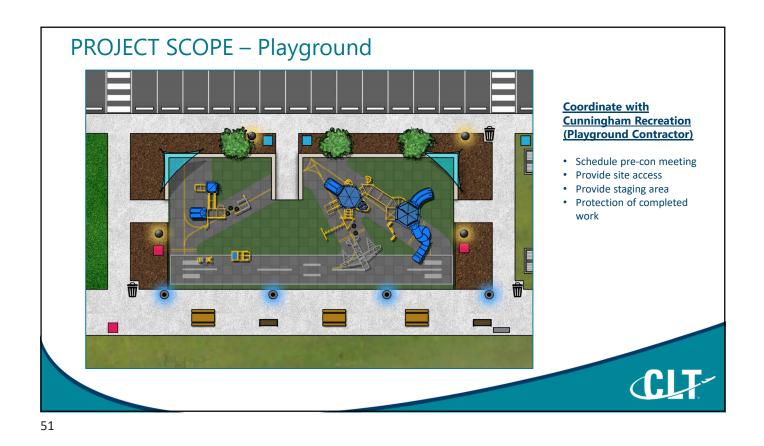


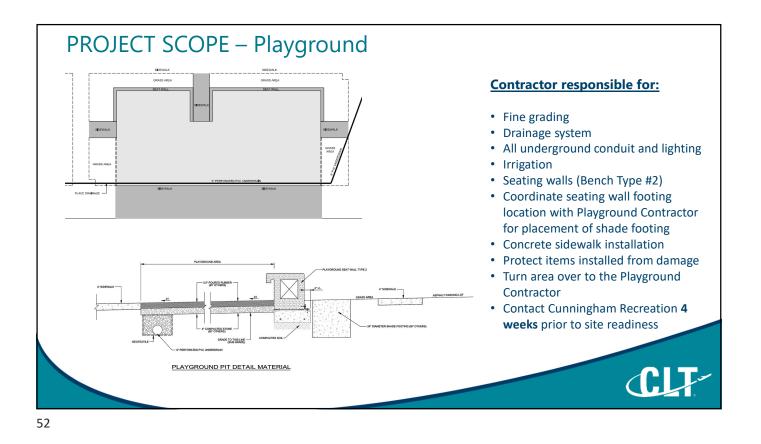


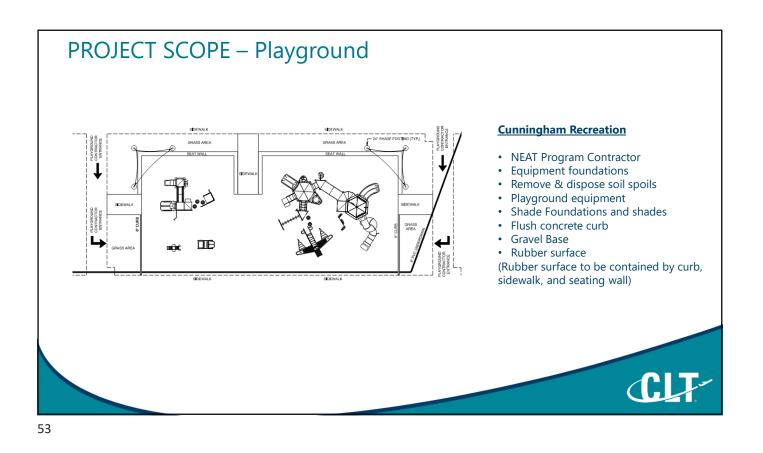




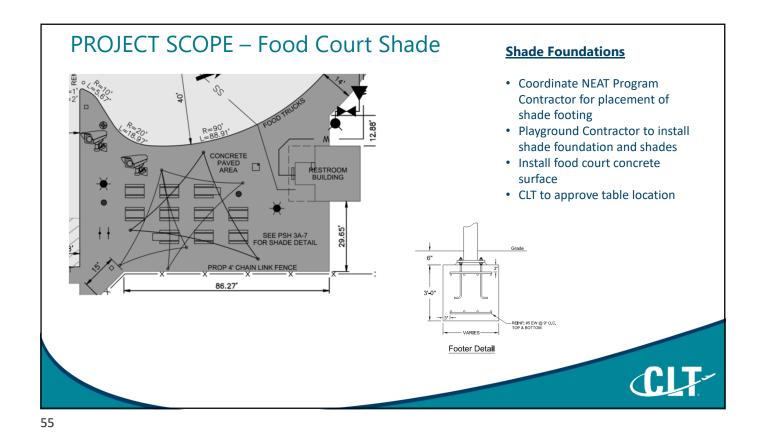


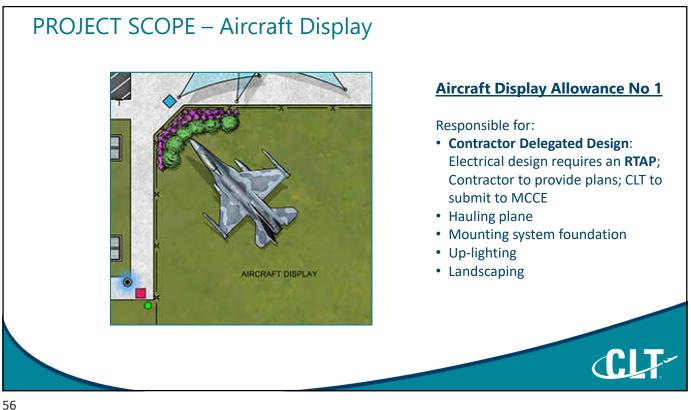


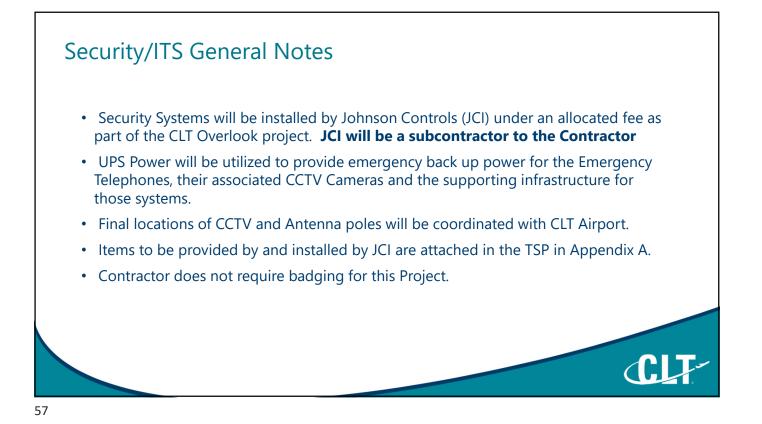


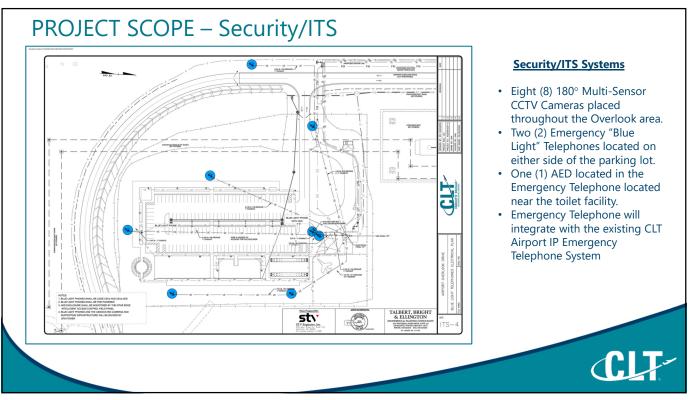


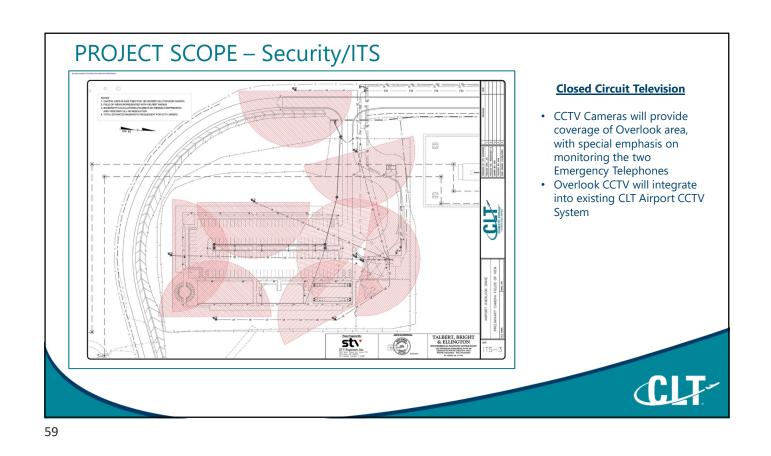


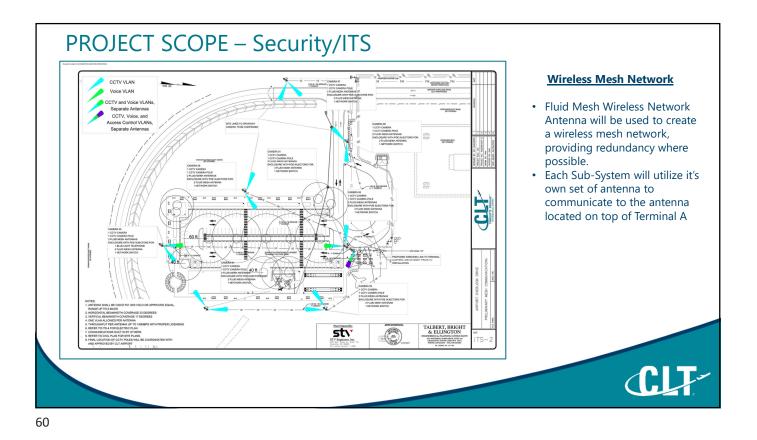




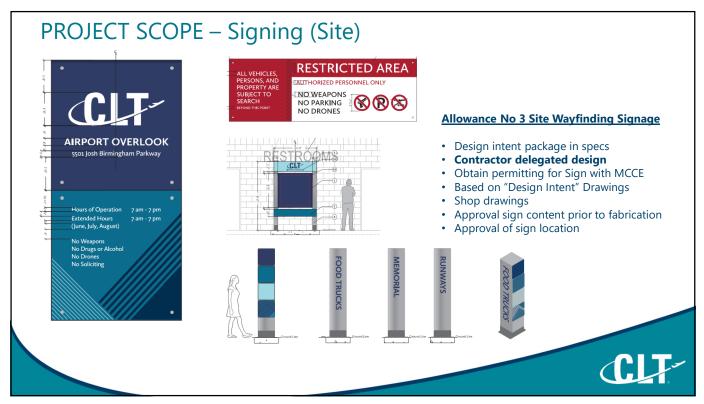


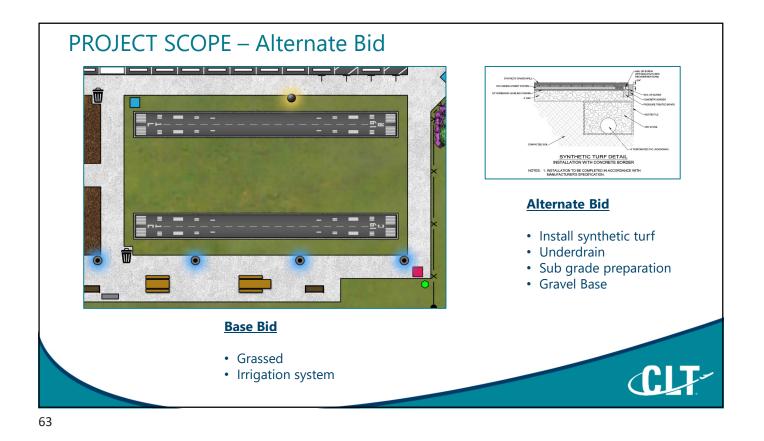








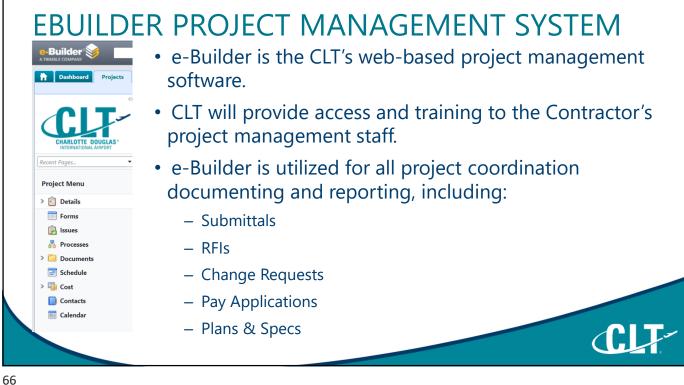


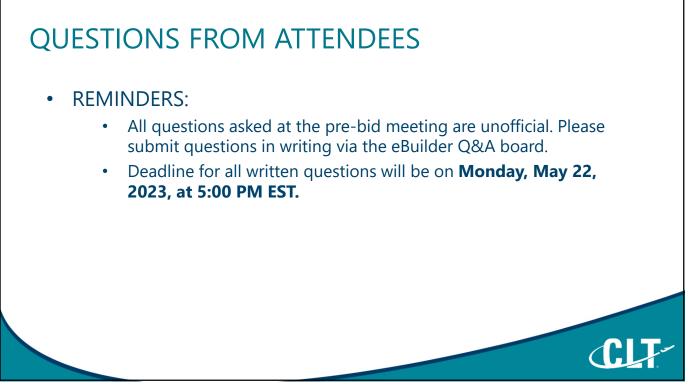


PROJECT CONTROLS REQUIREMENTS

- Contractor to staff appropriately for Project Controls functions including schedule, cost reporting and risk analysis.
- Provide reporting as detailed in Project Controls General Requirements
 - Example templates for Contractor use will be included via Addendum
 - Reports submitted via e-Builder process
- P6 CPM Schedule
 - Within 30 days of Contract Effective Date
- Progress Meetings
- Contractor's Financial Information Requirements

Description	Template Name	ELIVERABLES MATRIX Initial Submission for Acceptance	Owner Acceptance Timeline	Update / Distribution	
		General	Limeline		
Baseline Schedule (XER & PDF)	nʻs	Within 30 Days after Effective Date of Contract Approval	2 weeks after submitted	Until Baseline Schedule is Accepted/As Necessary for Revisions	
Baseline Schedule - Critical Path (PDF)	nʻa	Within 30 Days after Effective Date of Contract Approval	2 weeks after submitted	Until Baseline Schedule is Accepted/As Necessary for Revisions	
Baseline Schedule Acceptance Form	BL Acceptance	Within 30 Days after Effective Date of Contract Approval	2 weeks after submitted	7 Days after approved change	
Risk Register	Risk Register	Within 30 Days after Effective Date of Contract Approval	2 weeks after submitted	Monthly (2)	
Staffing Plan of General Conditions (Excel & PDF)	Staffing Plan	Within 30 Days after Effective Date of Contract Approval	2 weeks after submitted	Monthly (2)	
Monthly Progress Report	n'a	Within 30 Days after Effective Date of Contract Approval	2 weeks after submitted	Monthly (2)	
Weather Tracking Log	Weather Tracking Log	Within 30 Days after Effective Date of Contract Approval	2 weeks after submitted	Monthly (2)	
Procurement Status Report	Procurement Status Report	Within 30 Days after Effective Date of Contract Approval	2 weeks after submitted	Weekly (1)	
Full Schedule (XER & PDF)	nia	Prior to first Pay Application Submission	2 weeks after submitted	Monthly (2)	
Critical Path Schedule (PDF)	n'a	Prior to first Pay Application Submission	2 weeks after submitted	Monthly (2)	
Milestone Variance Report	Milestone Variance	Prior to first Pay Application Submission	2 weeks after submitted	Monthly (2)	
Schedule Change Log	Sch Change Log	Prior to first Pay Application Submission	2 weeks after submitted	Monthly (2)	
Schedule Adherence Report	Sch Adherence	Prior to first Pay Application Submission	2 weeks after submitted	Monthly (2)	
4 Week Lookahead Schedule (P6 PDF & Contractor Excel)	4 Weck	2 weeks prior to Preconstruction Meeting	2 weeks after submitted	Weekly (1) and Monthly (2)	
Contractor Daily Report	Daily Report	2 weeks prior to Preconstruction Meeting	2 weeks after submitted	Daily	
Safety Statistics Report	Safety Statistics Weekly Safety Statistics Monthly	2 weeks prior to Preconstruction Meeting	2 weeks after submitted	Weekly (1) and Monthly (2)	
Environmental Report	Environmental	First Week on Site	First Week on Site	Weekly (1) and Monthly (2)	
QA/QC Non-Conformance Log	QA_QC Non-Conformance Log	First Week on Site	First Week on Site	Weekly (1) and Monthly (2)	
Other Reports reasonably requested by Owner	nù	As Requested	As Requested	As Requested	
		Financial			
Schedule of Values	n'a	Within 30 Days after Effective Date of Contract Approval	2 weeks after submitted	Monthly	
Cash Flow Plan	Cash Flow	Prior to first Pay Application Submission	2 weeks after submitted	Monthly (2)	
Summary of CBI/MWSBE Tracking	CBI	Prior to first Pay Application Submission	2 weeks after submitted	Monthly (2)	
Change Order Log	CO Log	First Week on Site	2 weeks after submitted	Weekly (1) and Monthly (2)	
Pay Application - shall include Monthly Progress Schedule Update, Monthly Progress Report, Updated Schedule of Values, Updated Cash Flow Plan	Pay App Summary CBI MWSBE Tracking	On or before the tenth (10) working day		per contract	









CHARLOTTE DOUGLAS INTERNATIONAL AIRPORT CHARLOTTE, NORTH CAROLINA

ADDENDUM NO. 1

NORTH END AROUND TAXIWAY (NEAT) OVERLOOK RELOCATION

CLT PROJECT NO. AVIA 23-50 ADDENDUM DATE: MAY 16, 2023

PLAN HOLDERS LIST

Contact	Company	Email	Bid / Status
Hailie Enriquez	Archangel Protective Services	hailie.enriquez@archangelusa.com	No Pending
Logan Childs	Archer Western	lchilds@walshgroup.com	No Pending
Adam McDowell	Axiom Foundations, Inc.	amcdowell@axiomfoundations.com	No Pending
Eve Hibbler	Blythe Development	ehibbler@blythedevelopment.com	No Pending
Cauley Hobson	Chandler Construction Services, Inc.	chobson@chandlerconstruction.com	No Pending
EMILY GULLEDGE	Crowder Construction Company	egulledge@crowderusa.com	No Pending
daren daye	DMD Supply Services Group, LLC.	dmdaye@dmdsupplygroup.com	No Will bid
Kathy Marshall	Dodge Data & Analytics	kathy.marshall@construction.com	No Pending
Dave Scott	HALL CONTRACTING CORPORATION	dscott@hallcontracting.com	No Will bid
Derek Walker	Lane Construction	mdwalker@laneconstruct.com	No Will not bid
Lisa Markus	Martin Landscaping Co., Inc.	martinlandscaping@rocketmail.com	No Will bid
Will Gharst	MESSER CONSTRUCTION CO.	wgharst@messer.com	No Pending
Paul Winter	Neighboring Concepts	paul@neighboringconcepts.com	No Pending
Kara Ely	OnSite Development LLC	onsite17@outlook.com	No Pending
Freddie Young	Sanders Utility Construction Co INC	Freddie@sandersutility.com	No Pending
Walt Fisher	SHOWALTER CONSTRUCTION COMPANY INC	WFisher@showalterconstruction.com	No Pending
Scott Little	State Utility Contractors, Inc.	scott@sucontractors.com	No Pending

PROJECT RISK REGISTER SAMPLE										
PROJECT NAME	VERSION NUMBER		PROJECT START							
PROJECT MANAGER	DATE PREPARED		PROJECT FINISH							
OWNER	CONTRACT NUMBER		DURATION							

	LEGEND										
CATEGORY	LOW	MED	HIGH								
PROBABILITY	< 30%	30%><60%	>60%								
SCHEDULE IMPACT	< 1W	1W><3W	> 3W								
COST IMPACT	<\$100K	\$100K><\$500K	>\$500K								
RISK SCORE	<=3	3><7	> 7								

COST IMP		<\$100K	\$100K><\$500	Ж	>\$500K										
RISK SCOP	RE	<=3	3><7		>7										
										RISK REGISTER					
		RISK IDENTIFICATION RISK ASSESSMENT								ACTIO	N PLAN				
									1						
ID	DESCRIPTION	RISK	OWNER EXTERN	AL AGENCY	DATE REPORTED	UPDATED ACTION	PROBABILITY	SCHEDULE IMPACT	COST IMPACT	POTENTIAL IMPACT	RISK SCORE	ACTION PLANNED	STATUS	DATE ACHIEVED	ADDITIONAL COMMENTS
1							LOW	LOW	LOW		LOW		NOT STARTED		
$ \longrightarrow $															
.							LOW	LOW	LOW		LOW		NOT STARTED		
							2011	LOW	LOW		LOW		NOTSTARTED		
							LOW	LOW	LOW		LOW		NOT STARTED		
							LOW	LOW	LOW		LOW		NOT STARTED		
							LOW	LOW	LOW		LOW		NOT STARTED		
							2011		LOW		2000		NOTSTARTED		
							LOW	LOW	LOW		LOW		NOT STARTED		
												RISK REGISTER INSTRUCTIONS			
												RISK IDENTIFICATION			3
									RIPTION			of the potential risk to the project.			
									OWNER			anaging the risk on the Owners side of the pr			
									RNAL AGENCY	Date the risk is recorded		up responsible for the risk to the project outs	side of CLI's per	sonnei.	
									ATED ACTION	Date the risk is updated					
											0,1,3	RISK ASSESSMENT			
								PROE	BABILITY			or event occurring on the project. Please assig			
									DULE IMPACT			unt of time the risk will impacts the project's			
									IMPACT			ount the risk will have on the project. Please			
								POTE	NTIAL IMPAC			of the impacts to the project if the risk occur occurring on the project?	s. Will the risk i	mpact the projec	t schedule, project cost, or both
								RISK	SCORE			occurring on the project? ent based on the probability, schedule impact	t, and cost impa	ct of a risk to the	project. If the probability of a
												e is also likely HIGH. Please assign a risk score			
										(3><7), or HIGH (> 7).			CHARGE STORE		
												ACTION PLAN			
								ACTIO	ON PLANNED	Please provide a detaile	ed description	of the action planned by the project team to	mitigate the ris	sk to the project.	
								STAT	US	Please assign a status o	f the action pl	an to mitigate project risk. The categories are	NOT STARTED,	IN PROGRESS, or	COMPLETED.
								DATE	ACHIEVED	Date the action plan is a	ichieved.				

(CONTRACTOR NAME) Staffing Plan of General Conditions

1/30/2021

Project Name:

Date:

-

			JAN-21 Fi		FEB-21		MAR-21		
								Actual Time Assigned	
	Name	Rate (\$)	to This Project						
	John Smith	\$100		80%	100%		100%		
	John Smith	\$60	100%		100%		100%		
(Example) Project Controls	John Smith	\$30	100%	100%			50%		

		Weather Tracking Log		
Period	Expected "Weather Days" per Month	Non-Working "Weather Days" Allowance in the Schedule Calendar	Actual "Weather Days" Experienced	Expected vs Actual "Weather Days" - Variance
Jan-21	8	0	0	-8
Feb-21	7	0	0	-7
Mar-21	8	0	0	-8
Apr-21	6	0	0	-6
May-21	6	O	0	-6
Jun-21	6	0	0	-6
Jul-21	7	0	0	-7
Aug-21	6	0	0	-6
Sep-21	6	0	0	-6
Oct-21	5	0	0	-5
Nov-21	6	0	0	-6
Dec-21	6	0	0	-6

	Procurement Status Report												
Submittal ID	Major Procurement Items/Products	Submittal Date	Approval Date	P.O. Date	P.O. QTY	Expected Delivery Date	Actual Delivery Date	Estimated vs Actual Delivery Date Variance	Actual Delivered QTY	Pending QTY	Actual vs Pending QTY	Delayed/ Estimated Delivery Date	Reason for Delay
								0					
								0					
								0					
								0					
								0					
								0					
								0					
								0					
								0					
								0					
								0					
								0					
								0					
								0					
								0					
								0					

			Mile	stone Varian	ce Report				
Activity ID	Activity Name	Days from NTP	Contract	Baseline	Jan Update	Feb Update	Contract Variance	BL Variance	Jan vs Fe Variance
Intermediate	Milestones - Completion of a Phase, Completion of a Major Scope Item, and O	ther Key Dat	tes as Define	d by the Con	struction Do	cuments. Th	iis Also Includ	les Key Dates	of Importa
			0-Jan-00				0	0	0
			0-Jan-00				0	0	0
			0-Jan-00				0	0	0
			0-Jan-00				0	0	0
Owner Identi	ified Milestones - Key Dates for Operations, Security, and Other Coordination I	fforts Whick	h Will Be Prov	vided By CDI	A				•
			0-Jan-00				0	0	0
			0-Jan-00				0	0	0
			0-Jan-00				0	0	0
			0-Jan-00				0	0	0
			0-Jan-00				0	0	0

eb ce	Notes
ance	e To The Contractor.

	SCHEDULE	CHANGE LOG	6	
Total # of Activities	Not Started	In Progress	Completed	
			Number of	
	Descriptio		Activities	
	uration Change	S		
Baseline Dura	tion Changes			
			Number of	
	Descriptio	n	Activities	
Added Activiti				
Deleted Activi				
	Missed the La			
	Missed the La	te Finish Date		
Activities that	-			
Activities that	Finished Early			

	SCHEDULE ADHERENCE REPORT											
	Activities	s Planned	Actuals - Sta	rted/Finished								
Month	Start	Finish	Started	Finished	Calculation							

Steps

- 1. Contractor at the end of the previous month take a "snap shot" of the next months schedule (level 2 or 3)
- 2. Count the planned number of starts and finishes for the upcoming month.
- 3. Contractor performs work during the month and takes another "snap shot" of the schedule.
- 4. Count the completed number of starts and finishes that occurred during the month.
- 5. Run the following calculation each month to achieve the percentage % of actuals vs planned. (See example)
- 6. The outcome will help show "are you completing the activities you said you were going to do" Percentages should typically be over 75% to 80% to show schedule adherence.

Example:

Snap shot taken last closing cycle date of March 2020--> Snap Shot of April 2020 (next month) Snap shot shows planned starts = 20 and Finishes = 25 Snap shot taken at the end of the closing cycle for the current month ----> April 2020 Snap shot shows completed starts = 15 and finishes = 20 for the month of April 2020

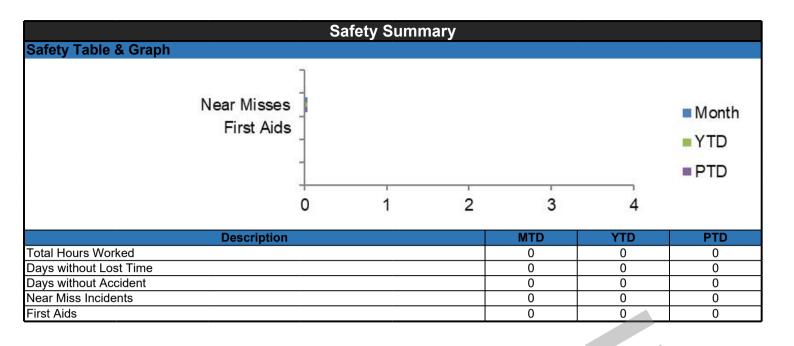
	Planned		Actual	l	
<u>Month</u>	<u>Start</u>	<u>Finish</u>	<u>Start</u>	<u>Finish</u>	
Apr-20	20	25	15	20	77.78%

Schedule Adherence = (Actual Starts & Actual Finishes) / (Planned Starts + Planned Finishes)

	Project Suj	ect Superintendent: Job Superintendent:																												
	3/1/2021									3/8/2021							3/15/2021						3/22/2021							
PROJECT:		SUN 3/1	MON 3/2	TUE 3/3	WED 3/4	THU 3/5	FRI 3/6	SAT 3/7	SUN 3/8												FRI 3/20			MON 3/23	TUE 3/24	WED 3/25	THU 3/26	FRI 3/27	SAT 3/28	
Project Days (580)			1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28
Phase/Sub Phase	Prime	Subcontractor																												
Phase 1																														
Example		Sub 1																												
Example	Prime																													
Example		Sub 1																												
Grade for Sod & Sod	Prime	Sub 2																												
Grooving Operation		Sub 5																												
RW CL fixtures and wire		Sub 3																												
B5 & B6 Plaza's, Conduit, Sign Pads		Sub 3																												
B-6 thru B-7		Sub 6																												
Lightning panel P-501		Sub 1																												
B10 to Echo TDZ and CL Fixtures		Sub 3																												
RW edge Wire		Sub 3																												
Video FPVC Runs		Sub 8																												
Stripe Phase 1		Sub 4																												
Vault Bussway		Sub 3																												
Generator Conduit		Sub 3																												
Pour Slabs and Mount Stairs on Generator		Sub 3																												
Russelectric Modify ATS		Sub 3																												
Phase 1E												· · · · ·													_					
Finish grade for/and Sod		Sub 2																												
Groove Concrete Panels		Sub 5																												
TW B10 Trim-Out (After surface treatment)		Sub 3																												
Surface Treatment		Sub 7																												
B-10 Stripe (west to east)		Sub 4																												

		CONTRACTOR DAILY REPORT			
		PROJECT NAME			
CONDITIONS					
Weather Conditions:	Example -> Sunny, Lo				
Site Conditions:		nditions, good, poor, flooding & etc.			
Rain Gauge Reading:	0.00"				
DAILY ACTIVITY INFORMATION	1				
Date:	Date of work				
Work Performed:	Subcontractor #1				
		pe - description of work performed during th	e work period. Include key		
	informati	on such as coordination with others.			
	Subcontractor #2				
		pe - description of work performed during th	e work period. Include key		
	informati	on such as coordination with others.			
Quantities:	Example: Clearing - 2				
Location:	Provide work areas f	or work performed above			
Deliveries:					
CONTRACTORS LABOR REPORT					
Contractors Personnel:		for each contractor & personell			
	Contractor:	# Superintendent, (#)Assistance Superinten			
		(#)laborers, (#)Technicians, (#)Painters, (#)F	-ire Protection & etc.		
	Dury isla labor warmen	for a sharehoust of the state o			
Subcontractor Personnel:	Provide labor report for each subcontractor & personell				
	Subcontractor:	# Superintendent, (#)Assistance Superinten			
		(#)laborers, (#)Technicians, (#)Painters, (#)	-ire Protection & etc.		
	Fuerrale 7.00	-00			
Contractor's Hours Worked:	Example 7:00 am - 5	:uu pm			
Visitors on Job-Site:	Example 2 FAA				
Accidents:	#				
CONTRACTOR'S MAJOR EQUIPM	1	Farring out if Other	1-11-		
Equipment On-Site	Quantity	Equipment, if Other,	Idle		
Provide Major Equipment -	1	Heavy Duty			
Crane					
ADDITIONAL COMMENTS					

Safe	ety Statistics Report - Weekly
a.	Safety Minute - Provide meeting topic to be discussed in Weekly Progress Meeting
b.	Man Hours to date
с.	First Aid Incidents this Week
d.	Near Misses this Week
e.	Recordable Incidents this Week
f.	Lost Time Accidents this Week
g.	Weeks Without Lost Time Accidents

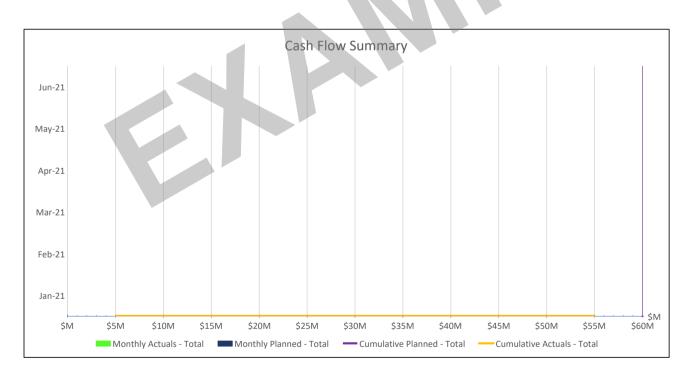


<u> </u>	Safety Statistics Report - Monthly				
		Data	Month	Year to Date project total for	Project to Date
				the current year (0	total project to
	Man hours	Man hours	reporting month	 12 months) project total for 	date
				the current year (0	total project to
	First Aid	First Aid	reporting month	 - 12 months) project total for 	date
				the current year (0	total project to
	Near Miss	Near Miss	reporting month	 - 12 months) project total for 	date
				the current year (0	total project to
	Recordables	Recordables	reporting month	 - 12 months) project total for 	date
				the current year (0	total project to
	Lost Time	Lost Time	reporting month	- 12 months)	date
	Project Recordable Incident Rate	0.0			
	National Average Rate	3.1			
	Project Lost Time Accidents				
	Project Lost Time Accident Rate	0.0			
	National Average	1.2			
I	Incident Review				
	Report on incidents that occurred in during the month	and reported above			
III	High Hazard Upcoming Activities		Risk Mitigation Tee	chniques	
vovide work sco	pe that is high hazard work upcoming	Provide risk mitigat	ion tools and techni	ques that will be used	to
on the project		prevent high hazar	d incidents		
V	Highlights				
	Provide any safety program highlights. Examples are S	afety Orientations,			
	Daily Stretch and Flex				

Date	Issue Description	Correction	Date Corrected
recipitation Table			
Date	Amount (inches)	Type (Rain, Snow, etc.)	Critical Path Schedu Impact Y/N
1/1/2021			
1/2/2021			
1/3/2021			
1/4/2021			
1/5/2021			
1/6/2021			
1/7/2021			
1/8/2021			
1/9/2021			
1/10/2021			
1/11/2021			
1/12/2021			
1/13/2021			
1/14/2021			
1/15/2021			
1/16/2021			
1/17/2021			
1/18/2021			
1/19/2021			
1/20/2021			
1/21/2021			
1/22/2021			
1/23/2021			
1/24/2021			
1/25/2021			
1/26/2021			
1/27/2021			
1/28/2021			
1/29/2021			
1/30/2021			
1/31/2021			

	Cash Flow Plan						
	Planned						
Month/Year	Early Finish	Late Finish	Average Monthly Total	Cumulative Total	Actual	Cumulative Total	
Jan-21			\$0	\$0		\$0	
Feb-21			\$0	\$0		\$0	
Mar-21			\$0	\$0		\$0	
Apr-21			\$0	\$0		\$0	
May-21			\$0	\$0		\$0	
Jun-21			\$0	\$0		\$0	
Total	\$0	\$0	\$0		\$0		

	· · · ·		Cash Flow	Plan			
Description	Jan-21	Feb-21	Mar-21	Apr-21	May-21	Jun-21	Total
Planned							
Early Finish							\$0
Late Finish							\$0
Average Monthly	\$0	\$0	\$0	\$0	\$0	\$0	\$0
Total							
Cummalative Total	\$0	\$0	\$0	\$0	\$0	\$0	
Actuals & Projections							
Monthly Total							
Cummalative Total	\$0	\$0	\$0	\$0	\$0	\$0	



Summary of CBI / MWSBE Tracking					
Description	MWSBE	Contract Value \$	Total MWSBE \$		
Contract Goals	0.00%				
Current Committtments	0.00%				
Current Status	0.00%				

				Poter	tial Change	Order Log					
CO #	Description	NOI Date	Pricing Proposal Amount	Schedule Impact Y/N	Days Requested	Date Sent to Owner	Owner Response Date	Approved Amount	Days Granted	Closed Y/N	Notes

Contractor's Certification of Buy American Compliance

[Contractor] hereby certifies that it has complied with 49 USC § 50101, BABA and other related U.S. statutes, guidance, and policies of the FAA by:

- a) Only installing iron, steel and manufactured products produced in the United States;
- b) Only installing construction materials defined as: an article, material, or supply other than an item of primarily iron or steel; a manufactured product; cement and cementitious materials; aggregates such as stone, sand, or gravel; or aggregate binding agents or additives that are or consist primarily of non-ferrous metals; plastic and polymer-based products (including polyvinylchloride, composite building materials, and polymers used in fiber optic cables); glass (including optic glass); lumber or drywall that have been manufactured in the United States.
- c) Installing manufactured products for which the Federal Aviation Administration (FAA) has issued a waiver as indicated by inclusion on the current FAA Nationwide Buy American Waivers Issued listing; or
- d) Installing products listed as an Excepted Article, Material or Supply in Federal Acquisition Regulation Subpart 25.104 and 25.108.

The attached documentation is provided as evidence of the source and origin of the iron, steel, and/or manufactured product, and certifying that these permanent construction materials used in the project are manufactured in the U.S.

Contract Pay Item #	Pay Item Description	Quantity	Product Manufacturer/Supplier*

Per 49 USC § 47126, this certification concerns a matter within the jurisdiction of the Federal Aviation Administration and the making of a false, fictitious or fraudulent certification may render the maker subject to prosecution under Title 18, United States Code.

Authorized Corporate Signature

Date

Printed Name

e-Mail Address

Position Title

Phone Number

* Attach the corresponding Manufacturer's Certification of Buy American Compliance for each Manufacturer or Supplier listed and <u>all</u> products listed on this sheet.

TO BE POPULATED ON MANUFACTURER OR SUPPLIER LETTERHEAD

Manufacturer's Certification of Buy American Compliance

This is to certify the items, products and/or materials listed below for use on the [Project Name] in [Project Location]

- a) are **wholly produced in the United States of 100% U.S. Material** in accordance with 49 USC § 50101; or
- b) are listed as a **Nonavailable Article in Federal Acquisition Regulation Subpart 25** and comply with 49 USC § 50101; or
- c) are included on the FAA Nationwide Buy American Waivers Issued list per 49 USC § 50101(b).

Part/Model #	Product Description	Quantity	Manufacturing Location

Per 49 USC § 47126, this certification concerns a matter within the jurisdiction of the Federal Aviation Administration and the making of a false, fictitious or fraudulent certification may render the maker subject to prosecution under Title 18, United States Code.

Authorized Corporate Signature

Date

Printed Name

e-Mail Address

Position Title

Phone Number

CHAIN LINK FENCE AND GATES



1.0 DESCRIPTION

Work covered by this special provision consists of furnishing and erecting chain link fence as shown on the plans and in accordance with the provisions of these specifications. This fencing is not intended to be SIDA fencing.

2.0 MATERIALS

2.1 Fabric. The fabric shall be woven with a 9-gauge galvanized steel wire in a 2-inch (50 mm) mesh and shall meet the requirements of ASTM A392, Class 2.

2.2 Posts, rails, and braces. Posts, rails, and braces shall conform to the requirements of ASTM F1043 or ASTM F1083 as follows:

• Galvanized tubular steel pipe shall conform to the requirements of Group IA, (Schedule 40) coatings conforming to Type A, or Group IC (High Strength Pipe), external coating Type B and internal coating Type B or D.

Posts, rails, and braces, with the exception of galvanized steel conforming to ASTM F1043, Material Group IA, Type A, shall demonstrate the ability to withstand testing in salt spray in accordance with ASTM B117 as follows:

- External: 1,000 hours with a maximum of 5% red rust.
- Internal: 650 hours with a maximum of 5% red rust. •

The dimensions of the posts, rails, and braces shall be in accordance with Tables I through IV of Federal Specification RR-F-191/3.

2.3 Gates. Gate frames shall consist of galvanized steel pipe and shall conform to the specifications for the same material under paragraph 2.2. The fabric shall be of the same type material as used in the fence.

- Gates shall be provided with galvanized malleable iron or heavy gauge post and frame hinges. •
- Single gates shall be provided with positive locking latches fabricated of 5/16 in. by 1-3/4 in pressed galvanized steel.
- One leaf of double gates shall have a positive anchor consisting of two galvanized malleable iron or heavy gauge brackets and a ¾ in. "L" shaped slide bar secured to a casting or embedded pipe in a 10 in. pier (minimum 18 in. embedment); the mating leaf shall have a positive locking latch described above.

- Each gate shall have a continuous concrete pad shown in the details extending to a point 1'-0" beyond gate piers on each side of the opening.
- The Owner will provide chains and padlocks at Project completion.

2.4 Wire ties and tension wires. Wire ties for use in conjunction with a given type of fabric shall be of the same material and coating weight identified with the fabric type. Tension wire shall be 7-gauge marcelled steel wire with the same coating as the fabric type and shall conform to ASTM A824. All material shall conform to Federal Specification RR-F-191/4.

2.5 Miscellaneous fittings and hardware. Miscellaneous steel fittings and hardware for use with zinccoated steel fabric shall be of commercial grade steel or better quality, wrought or cast as appropriate to the article, and sufficient in strength to provide a balanced design when used in conjunction with fabric posts, and wires of the quality specified herein. All steel fittings and hardware shall be protected with a zinc coating applied in conformance with ASTM A153. Barbed wire support arms shall withstand a load of 250 pounds (113 kg) applied vertically to the outermost end of the arm.

2.6 Concrete. Concrete shall have a minimum 28-day compressive strength of 3,000 psi (2670 kPa). Concrete for post footings shall be manufactured using Type III, High Early Strength Portland cement. Concrete used in pads under gates may use either Type I or Type III Portland cement at Contractor's option.

2.7 Marking. Each roll of fabric shall carry a tag showing the kind of base metal (steel), kind of coating, the gauge of the wire, the length of fencing in the roll, and the name of the manufacturer. Posts, wire, and other fittings shall be identified as to manufacturer, kind of base metal (steel), and kind of coating.

1.0 SUBMITTALS

Prepare and submit to the Engineer for approval complete shop drawings and design computations for the bracing, accessory hardware, and foundations for the 3-Bar Metal Gate and Double Swing Gate.

Have a professional engineer registered in the State of North Carolina, perform the computations, and render a set of sealed, signed and dated drawings detailing the construction of each gate.

4.0 CONSTRUCTION METHODS

4.1 General. The fence shall be constructed at locations shown on the plans and as specified here using new materials. The Contractor shall layout the fence line based on the plans. The Contractor shall span ditch openings below the fence as shown in the ditch crossing detail.

4.2 Clearing fence line. Clearing shall consist of the removal of all stumps, brush, rocks, trees, or other obstructions that will interfere with proper construction of the fence. Stumps within the cleared area of the fence shall be grubbed or excavated. The bottom of the fence shall be placed a uniform distance above ground, as specified in the plans. All holes remaining after post and stump removal shall be refilled with suitable soil, gravel, or other suitable material and compacted with tampers.

4.3 Installing posts. All posts shall be set in concrete at the required dimension and depth and at the spacing shown on the plans.

The concrete shall be thoroughly compacted around the posts by tamping or vibrating and shall have a smooth finish slightly higher than the ground and sloped to drain away from the posts. All posts shall be set plumb and to the required grade and alignment. No materials shall be installed on the posts, nor shall the posts be disturbed in any manner within twenty-four (24) hours after the individual post footing is completed.

Should rock be encountered at a depth less than the planned footing depth, a hole 2 inches (50 mm) larger than the greatest dimension of the posts shall be drilled to a depth of 12 inches (300 mm). After the posts are set, the remainder of the drilled hole shall be filled with grout, composed of one part Portland cement and two parts mortar sand. Any remaining space above the rock shall be filled with concrete in the manner described above.

In lieu of drilling, the rock may be excavated to the required footing depth. No extra compensation shall be made for rock excavation.

4.4 Installing top rails. The top rail shall be continuous and shall pass through the post tops. The coupling used to join the top rail lengths shall allow for expansion.

4.5 Installing braces. Horizontal brace rails, with diagonal truss rods and turnbuckles, shall be installed at all terminal posts.

4.6 Installing fabric. The wire fabric shall be firmly attached to the posts and braced as shown on the plans. All wire shall be stretched taut and shall be installed to the required elevations. The fence shall generally follow the contour of the ground, with the bottom of the fence fabric no less than one inch (25 mm) or more than 4 inches (100 mm) from the ground surface. Grading shall be performed where necessary to provide a neat appearance.

At locations of small natural swales or drainage ditches and where it is not practical to have the fence conform to the general contour of the ground surface, longer posts may be used and multiple strands of barbed wire stretched to span the opening below the fence as shown in the details. The vertical clearance between strands of barbed wire shall be 6 inches (150 mm) or less.

5.0 MEASUREMENT

5.1 The quantity of chain link fence to be measured is the actual number of linear feet of fence, measured from center of post to center of post, which has been completed and accepted.

5.2 Gates will be measured as complete units.

Double swing gate will be measured per each. This pay item includes all work associated with the gate, including but not limited to, the gate, sign placement, and all items for the gate to function.

6.0 PAYMENT

6.1 Payment for chain link fence will be made at the contract unit price per linear foot. No measure or payment will be made for ditch crossings. The ditch crossings will be incidental and included with the payment for chain link fence.

6.2 Payment for vehicle gates will be made at the contract unit price for each gate.

The price shall be full compensation for furnishing all materials, and for all preparation, erection, and installation of these materials, and for all labor equipment, tools, and incidentals necessary to complete the item.

Payment will be made under:

FENCE, 96" CHAIN LINK	LF
28' 96"- CHAIN LINK DOUBLE SWING GATE (Overlook Site)	
30' 3-BAR METAL GATE (Food Truck Drive)	EA
32' DOUBLE SWING GATE (End of Overlook Drive & ASR Entrance)	EA

AIRCRAFT DISPLAY (ALLOWANCE NO. 1)



1.0 GENERAL

The contractor shall construct an aircraft display at the location designated in the plans and CLT will provide the current location of the aircraft. The contractor will be responsible for hauling the aircraft to the site (F-14 aircraft located within 75 miles of CLT) installing the aircraft and LED display lighting.

The Aircraft Display Allowance is to provide payment for this Work. Any unused portion of the allowance remaining at the completion of the Contract shall revert to the Owner as a credit.

An example of the work required:





1.1 COORDINATION

At the earliest practical date after award of the Contract, the Contractor shall advise Owner of the date when final selection and purchase of each product or Work described by an Allowance must be completed to avoid delaying the Work.

1.2 PROCEDURES

Coordinate and process submittals for additional work for the Aircraft Display Allowance. Contractor shall submit a detail cost proposal for purchase of products and work for review and approval prior to commencing work.

2.0 EXECUTION

SCHEDULE OF ALLOWANCES

The aircraft shall be furnished by the Owner and the Contractor shall haul and install the aircraft at the location shown on the plan.

The Contractor shall provide all labor and materials to provide complete display installation of the aircraft including, but not limited to, hauling, aircraft, anchors, wheel stops, and display lighting (including MCCE permits, and additional conduit and conductors not shown on plan).

Preparation of the display location, grading, fencing, conduit, handholds, landscaping, foundation, and electrical conductors as show on the plans will be provided by their separate pay items.

3.0 PAYMENT

An amount has been designated by the Owner to approximately represent Contractor's cost for all labor and material to install the Aircraft Display item(s) and any applicable sales tax. This amount, at its final accounting, could be more or less depending on Owner's selection(s) and/or a third party's actual charges to the CONTRACTOR.

If the cost of the allowance amount exceeds the amount specified, then that amount will be added to the next progress payment or final payment as approved by CLT. If the amount is less than the allowance amount, then that amount will be subtracted from the final amount of the Contract.

ALLOWANCE NO. 1 AIRCRAFT DISPLAY	/ ALLOWANCE
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BOLLARD AND CHAIN BARRIER



1.0 DESCRIPTION

The bollard and chain barrier are to designate the boundary of the aircraft display and create a deterrent for safety.

Section shall consist of furnishing and erecting Manufactured Metal Bollards and Chains in accordance with these specifications, the details shown on the plans, and in conformity with the lines and grades shown on the plans. The Bollard Chain barrier installation includes all bollards, quick release opening, and concrete foundation.

1.1 SUBMITTALS

- A. Comply with shop drawing Submittal Procedures
- B. Product Data submit the following information:
 - 1. Product literature
 - 2. CAD drawings
 - 3. Made in the USA Statement
 - 4. Proprietary information
- D. Warranty submit manufacturer's standard warranty

1.2 QUALITY ASSURANCE

A. Manufacturer's Qualifications

1.3 DELIVERY, STORAGE AND HANDLING

- A. Delivery: Deliver products to site in manufacturer's clearly marked original, unopened containers and packaging. Upon delivery, examine packages immediately to ensure all products are complete and undamaged.
- B. Storage: Store products in a protected, dry area in manufacturer's unopened containers and packaging until installation.
- C. Handling: Protect product's finish from damage during handling and installation.

1.4 WARRANTY

- A. Material is warranted against material and workmanship defects for THREE YEARS from the date of shipment from manufacturer, assuming products are installed and maintained according to manufacturer's instruction.
- B. Products damaged by abnormal use, vandalism or acts of nature are not eligible for this factory warranty.

C. Manufacturer will repair or replace any part found defective upon written notification and inspection by our manufacturer's representative.

PART 2 - PRODUCTS

2.1 BOLLARD

- A. Permanent metal bollard 36" Minimum height above grade
- B. Fully assembled metal powder coated bollard
- C. Made in the USA
- D. Complete weld seams where two pieces of metal touch to reduce chance of corrosion
- E. Concrete foundation
- F. Metal powder coated in black color

2.2 CHAIN

- A. Link bollards with 5/16" connecting chaiins to deter pedestrias from entering the aircraft display area. Attach chains with chain eyes. Provide one quick link for easy removal to provide access.
- B. Chains are to be black powder coated galvanized steel.

2.3 MATERIALS

- A. Commercial-grade materials,
- B. Completely welded for optimum strength and stability.
- C. Sizes and dimensions according to Manufacture recommendation
- D. Minimum 4" schedule 40 pipe with half dome cap; 11 ga x 3-3/4" tubing removable sleeve
- E. 1/4 x 2" flat metal tabs for concrete stabilization
- F. 1/2" cast iron d-rings for chain loops

2.4 FINISHES

Metal products are finished with a two-coat powder coating process applied to a 7-15 mil thickness. Substrate preparation includes sandblasting to remove all surface contaminants, place a corrosion-inhibiting coating prior to the application of the powder coating. The first coat applied to the substrate is zinc rich epoxy powder primer used exclusively on sandblasted parts. The second coat is a colored polyester powder coating. Both coats are electrostatically applied, and oven cured according to powder coating manufacturing specifications to create a smooth, black satin-like finish.

2.5 FASTENERS

Provide fasteners of size according to the manufacture's recommendation.

PART 3 – EXECUTION

3.1 EXAMINATION

Confirm that installation area is as shown in the plans and bollard locations are approved by the Engineer for product placement.

3.2 INSTALLATION

- A. Install product in accordance with manufacturer's instructions at locations indicated on the drawings.
- B. Install product level and plumb.
- C. Anchor product securely in place.

3.3 PROTECTION

Protect products prior to installation by having them remain in the manufacturer's packaging and container.

PART 4 – MEASUREMENT

Bollard and Chain Barrier will be measured for payment by the number of bollards installed and accepted. No separate measurement will be made for concrete foundations, chain, and connections. Payment for these items is incidental to the measurement for *Bollard and Chain Barrier*.

PART 5 - PAYMENT

Payment for *Bollard and Chain Barrier* will be made at the contract unit price per each.

The price shall be full compensation for furnishing all materials, and for all preparation, erection, and installation of these materials, and for all labor equipment, tools, and incidentals necessary to complete the item.

AIRCRAFT DISPLAY CONCRETE FOUNDATION:

1.0 Description

The work covered by this special provision includes furnishing all materials, labor, equipment, and incidentals necessary to install Cast-in-place footing for Aircraft Display in accordance with these provisions and contract documents.

2.0 MATERIALS

Refer to NCDOT Standard Specifications Division 10.

Item	Section
Reinforcing Steel	1070
Portland Cement Concrete	1000

3.0 MEASUREMENT AND PAYMENT

All items associated with the *Airplane Display Concrete Foundation* will be paid for at the contract lump sum price for *Airport Display Concrete Foundation*. Such Price and payment will be full compensation for all work covered by this special provision, the plans and applicable parts of the NCDOT standard specifications and will include, but not be limited to, furnishing all labor, materials, equipment and other incidentals necessary to complete this work. Such price and payment will also be full compensation for concrete, reinforcing steel, excavating, backfilling, labor and all other related materials necessary for the completion of the footing. No separate measurement or payment will be made for items needed to complete the work satisfactorily.

Payment will be made under:

Aircraft Display Concrete Foundation LS



A. ITEMIZED BID

NEAT OVERLOOK RELOCATION

Charlotte Douglas International Airport Project No.: **AVIA 23-50**

BASE BID (Unit Price Total Amount + Allowances = BASE BID)

The undersigned Bidder, having carefully examined the Bidding and Contract Documents, and having visited the site, and being familiar with all conditions and requirements of the Work, hereby agrees to furnish all material, labor, equipment, permits and services, including all scheduled Allowances, necessary to complete the Work for the above-named project, in accordance with the requirements of the Bidding Documents, for the sum of: Dollars (\$_____)

ALTERNATES (Base Bid + Allowances + Alternate(add/deduct) = ALTERNATES)

The undersigned Bidder, having carefully examined the Bidding and Contract Documents, and having visited the site, and being familiar with all conditions and requirements of the Work, hereby agrees to furnish all material, labor, equipment, permits and services, including all scheduled Allowances and Alternate scope, necessary to complete the Work for the above-named project, in accordance with the requirements of the Bidding Documents, for the sum of: _______Dollars (\$______)

UNIT PRICES

Line Item	Spec No.	Description	Qty	Unit	Unit Price	Amount
1	800	MOBILIZATION (5%)	1	LS		
2	200	CLEARING AND GRUBBING	1	LS		
3	200	SUPPLEMENTARY CLEARING AND GRUBBING	1	ACR		
4	225	UNCLASSIFIED EXCAVATION	32500	CY		
5	225	UNDERCUT EXCAVATION	250	CY		
6	235	EMBANKMENT SETTLEMENT GAUGES	8	EA		
7	260	PROOF ROLLING	20	HR		
8	270	GEOTEXTILE FOR SOIL STABILIZATION	6000	SY		
9	300	FOUNDATION CONDITIONING MATERIAL, MINOR STRUCTURES	150	TN		
10	300	FOUNDATION CONDITIONING GEOTEXTILE	500	SY		
11	310	15" RC PIPE CULVERTS, CLASS III	276	LF		
12	310	18" RC PIPE CULVERTS, CLASS III	292	LF		
13	310	24" RC PIPE CULVERTS, CLASS IV	64	LF		
14	310	15" RC PIPE CULVERTS, CLASS IV	60	LF		
15	310	18" RC PIPE CULVERTS, CLASS IV	104	LF		
16	300	6" CLEAN OUT	2	EA		
17	500	FINE GRADING	1	LS		
18	520	AGGREGATE BASE COURSE	9,000	TN		
19	560	SHOULDER BORROW	50	CY		
20	607	INCIDENTAL MILLING	100	SY		
21	610	ASPHALT CONC INTERMEDIATE COURSE, TYPE I19.0B	1,500	TN		

Line Item	Spec No.	Description	Qty	Unit	Unit Price	Amount
22	610	ASPHALT CONC SURFACE COURSE, TYPE S9.5B	2000	TN		
23	620	ASPHALT BINDER FOR PLANT MIX	200	TN		
24	654	ASPHALT PLANT MIX, PAVEMENT REPAIR	30	TN		
25	815	6" PVC SUBDRAIN, SCHEDULE 80	500	LF		
26	840	MASONRY DRAINAGE STRUCTURES	6	EA		
27	840	FRAME WITH GRATE, STD 840.14	2	EA		
28	840	FRAME WITH COVER, STD 840.54	4	EA		
29	846	2'-6" CONCRETE CURB AND GUTTER	100	LF		
30	SP	FLUSH CONCRETE CURB	300	LF		
31	848	4" CONCRETE SIDEWALK	4900	SY		
32	SP	FENCE, 48" CHAIN LINK	0	LF		
33	SP	FENCE, 96" CHAIN LINK	2500	LF		
34	SP	28' 96"- CHAIN LINK DOUBLE SWING GATE (Overlook Site)	1	EA		
35	SP	12' 48"- CHAIN LINK DOUBLE SWING GATE (Aircraft Display)	0	EA		
36	SP	30' 3-BAR METAL GATE (Food Truck Drive)	1	EA		
37	SP	32' DOUBLE SWING GATE (End of Overlook Drive & ASR Entrance)	2	EA		
38	SP	REMOVAL OF EXISTING FENCE	500	LF		
39	876	RIP RAP, CLASS B	5	TN		
40	876	GEOTEXTILE FOR DRAINAGE	10	SY		
		SIGNING				
41	901	CONTRACTOR FURNISHED, TYPE E SIGN	168	SF		
42	903	SUPPORTS, 3-LB STEEL U-CHANNEL	860	LF		
43	904	SIGN ERECTION TYPE E (GROUND MOUNTED)	64	EA		
		TRAFFIC CONTROL			· · · · · ·	
44	1110	WORK ZONE SIGNS (STATIONARY)	54	SF		
45	1110	WORK ZONE SIGNS (PORTABLE)	54	SF		
46	1110	WORK ZONE SIGNS (BARRICADE MOUNTED)	10	SF		
47	1130	DRUMS	100	EA		
48	1145	BARRICADES (TYPE III)	100	LF		
49	1150	FLAGGER	6	DAY		
		PAVEMENT MARKING			· · · · · ·	
50	1205	THERMOPLASTIC PAVEMENT MARKING LINES (4",90 MIL)	12,000	LF		
51	1205	THERMOPLASTIC PAVEMENT MARKING LINES (8",90 MIL)	50	LF		
52	1205	THERMOPLASTIC PAVEMENT MARKING LINES (24" 90 MIL)	100	EA		
53	1205	THERMOPLASTIC PAVEMENT MARKING SYMBOL (90 MIL)	4	EA		
54	1205	PAINT PAVEMENT MARKING LINES (4")	4,000	LF		
55	1205	PAINT PAVEMENT MARKING LINES (12")	600	LF		
56	1205	PAINT PAVEMENT MARKING LINES (24")	650	LF		

Line Item	Spec No.	Description	Qty	Unit	Unit Price	Amount
57	1205	PAINT PAVEMENT MARKING SYMBOL	16	EA		
		SITE AMENITIES	•			
58	SP	ALLOWANCE NO.1 AIRCRAFT DISPLAY	1	ALLOW	125,000.00	125,000.00
59	SP	ALLOWANCE NO.2 AIRPORT OVERLOOK ENTRANCE SIGN	1	ALLOW	150,000.00	150,000.00
60	SP	ALLOWANCE NO.3 SITE WAYFINDING SIGNAGE	1	ALLOW	115,000.00	115,000.00
61	SP	AIRPORT OVERLOOK ENTRANCE SIGN CONCRETE FOUNDATION	1	LS		
62	SP	MOCK RUNWAY MARKING	1	LS		
63	SP	REMOVABLE STEEL BOLLARD	5	EA		
64	SP	CONCRETE WHEEL STOP	155	EA		
65	SP	SYNTHETIC TURF	7,500	SF		
66	SP	8' SURFACE MOUNT PICNIC TABLE	23	EA		
67	SP	8' ACCESSIBLE SURFACE MOUNT PICNIC TABLE	6	EA		
68	SP	6' BENCH SEAT (CLT SUPPLIED)	12	EA		
69	SP	TRASH RECEPTACLE	12	EA		
70	SP	PET WASTE STATION	4	EA		
71	SP	RACK FOR BICYCLE PARKING, CLDS 50.21	3	EA		
72	SP	TRASH DUMPSTER ENCLOSURE	1	LS		
		SITE ELECTRICAL AND LIGHTING		-		
73	SP	CONCRETE BOLLARD LIGHTING	12	EA		
74	SP	ELECTRICAL JUNCTION BOX	20	EA		
75	SP	6" ELECTRICAL CONDUIT, SCHEDULE 40	700	LF		
76	SP	3" ELECTRICAL CONDUIT, SCHEDULE 40	5,500	LF		
77	SP	2" ELECTRICAL CONDUIT, SCHEDULE 40	300	LF		
78	SP	1" ELECTRICAL CONDUIT, SCHEDULE 40	1,000	LF		
79	SP	2 AWG SIZE 8 CONDUCTOR (BK & RD)	3,000	LF		
80	SP	1 AWG SIZE 10 GROUNDING CONDUCTOR	2,000	LF		
81	SP	100 Amps, 208Y/120/240V LOAD CENTER	1	EA		
		SITE SECURITY				
82	SP	ALLOWANCE NO.4 SECURITY ITEMS PROVIDED, INSTALLED AND ITEGRATED BY JCI	1	Allowance	314,000.00	314,000.00
83	SP	NETWORK SWITCH (TERMINAL COMMUNICATIONS ROOM)	8	EA		
84	SP	1" CONDUIT (MATERIAL AND INSTALLATION)	3,500	LF		
85	SP	2 AWG SIZE 8 CONDUCTOR (BK & RD)	5,000	LF		
86	SP	1 AWG SIZE 10 GROUNDING CONDUCTOR	3,500	LF		
87	SP	CAT-6	100	LF		
88	SP	24 VDC POWER SUPPLIES	11	EA		
89	SP	48 VDC POWER SUPPLIES	8	EA		
90	SP	POE INJECTOR	1	EA		
91	SP	EQUIPMENT ENCLOSURE	8	EA		
92	SP	PELCO 12' POLE	8	EA		
93	SP	PELCO POLE BASE	8	EA		

Line Item	Spec No.	Description	Qty	Unit	Unit Price	Amount
94	SP	ANCHOR BOLTS	64	EA		
95	SP	ACORN CAP FOR POLE	8	EA		
96	SP	FIRE DEPARTMENT ACCESS KEY LOCK BOX	1	EA		
		WATER AND SEWER				
97	SPUC- 01A	8" DI PC350 RESTRAINED JOINT WATER LINE W/FITTINGS	2,010	LF		
98	SPUC- 01B	6" DI PC350 RESTRAINED JOINT WATER LINE W/FITTINGS	74	LF		
99	SPUC- 01C	2" PVC SCH40 WATER LINE W/FITTINGS	2,000	LF		
100	SPUC- 01D	1" PVC SCH40 WATER LINE W/FITTINGS	1,800	LF		
101	SPUC- 02A	8" GATE VALVE	2	EA		
102	SPUC- 02B	6" GATE VALVE	5	EA		
103	SPUC- 02C	MJ CAP W/ 2" BLOW OFF ASSEMBLY	1	EA		
104	SPUC- 02D	1" IRRIGATION WATER SERVICE METER WITH BACKFLOW	1	EA		
105	SPUC- 02E	2" DOMESTIC WATER METER WITH BACKFLOW	1	EA		
106	SPUC- 02F	8" FIRE LINE SERVICE WITH DETECTOR METER AND BACKFLOW	1	EA		
107	SPUC- 02G	FIRE HYDRANT ASSEMBLY	4	EA		
108	SPUC- 02H	24" X 8" TAPPING SADDLE AND VALVE	1	EA		
109	SPUC- 03A	6" SEWER SERVICE WITH CLEANOUTS	294	LF		
110	SPUC- 03B	PRECAST SEPTIC TANK 2500 GALLONS	1	EA		
111	SPUC- 03C	PRECAST PUMP TANK W/ PUMP SYSTEM AND CONNECTION BETWEEN TANKS	1	EA		
112	SPUC- 03D	2" FORCE MAIN WITH CLEANOUTS, BALL VALVES AND PVC SLEEVES	2,591	LF		
113	SPUC- 03E	AIR RELEASE VALVE ASSEMBLY	2	EA		
114	SPUC- 03F	MANIFOLD ASSEMBLY W/SUPPLY LINES	1	EA		
115	SPUC- 03G	NITRIFICATION LINES INCLUDING CLEARING AND ALL SEPTIC FIELD RESTORATION	1,500	LF		
116	SPUC- 04A	REMOVAL OF UNSUITABLE MATERIAL	150	СҮ		
117	SPUC- 04B	SELECT FILL	150	CY		
		BUILDING AND ARCHITECTURAL	1		-	
118	SP	ARCHITECTURAL	1	LS		
119	SP	STRUCTURAL	1	LS		
120	SP	PLUMBING	1	LS		
121	SP	MECHANICAL	1	LS		
122	SP	ELECTRICAL	1	LS		

Line Item	Spec No.	Description	Qty	Unit	Unit Price	Amount
123	SP	CONCRETE PAVERS (MEMORIAL)	1	LS		
124	SP	PRE-CAST BENCH TYPE 1 (MEMORIAL)	1	LS		
125	SP	PRE-CAST BENCH TYPE 2 (PLAYGROUND)	1	LS		
		LANDSCAPING				
126	SP	SHRUB - 3 GAL	217	EA		
127	SP	TREE - 2" CAL MIN.	27	EA		
128	SP	MULCH	100	CY		
129	SP	TOP SOIL	650	CY		
130	SP	RIVER ROCK	100	TN		
131	SP	IRRIGATION SYSTEM - COMPLETE	1	LS		
		EROSION CONTROL			<u> </u>	
132	1605	TEMPORARY SILT FENCE	5,000	LF		
133	1610	EROSION CONTROL STONE, CLASS B	400	TN		
134	1615	TEMPORARY MULCHING	120	ACR		
135	1620	SEED FOR TEMPORARY SEEDING	500	LB		
136	1620	FERTILIZER FOR TEMPORARY SEEDING	3	TN		
137	1622	TEMPORARY SLOPE DRAINS	700	LF		
138	1630	SILT EXCAVATION	3,000	CY		
139	1631	MATTING FOR EROSION CONTROL	13,000	SY		
140	SP	COIR FIBER MAT	20	SY		
141	SP	PERMANENT SOIL REINFORCEMENT MAT	1,500	SY		
142	1632	1/4" HARDWARE CLOTH	200	LF		
143	SP	24" TEMPORARY PIPE	300	LF		
144	SP	COIR FIBER WATTLE	300	LF		
145	SP	SKIMMER BASIN MAINTENENANCE	2	EA		
146	1660	SEEDING AND MULCHING	4	ACR		
147	1660	MOWING	4	ACR		
148	1661	SEED FOR REPAIR SEEDING	100	LB		
149	1661	FERTILIZER FOR REPAIR SEEDING	1	TN		
150	1662	SEED FOR SUPPLEMENTAL SEEDING	100	LB		
151	1665	FERTILIZER TOPDRESSING	2	TN		
152	SP	CONCRETE WASHOUT STRUCTURE	3	EA		
153	1667	SPECIALIZED HAND MOWING	20	MHR		
		OVERLOOK WATER QUALITY BASIN				
154	200	Borrow Excavation	1,037	CY		
155	225	No. 57 Washed Stone for Sand Filters	333	TN		
156	225	6" Perforated PVC Pipe	414	LF		
157	225	8" Non-Perforated PVC Pipe	94	LF		
158	275	Geotextile Fabric for Filter Basin	997	SY		
159	520	PVC Cleanout	16	EA		
160	876	PVC Elbows	2	EA		
161	876	PVC Tees	4	EA	1	

Line Item	Spec No.	Description	Qty	Unit	Unit Price	Amount
162	1620	Pre-Caste Riser Basin	1	EA		
163	1660	Pre-Caste Riser Basin (Above 5' VF)	1	EA		
164	838	4" DIP	2	LF		
165	838	4" Gate Valve and Valve Box	1	EA		
166	825	HDPE Peaked Trash Rack	1	EA		
167	310	Rip Rap, Class I	17	TN		
168	310	Rip Rap, Class A	321	TN		
169	825	Geotextile Fabric, Type-2	466	SY		
170	1605	Temporary Silt Fence	476	LF		
171	1605	Inlet Protection	6	EA		
172	1639	Special Stilling Basins	1	EA		
173	1664	Sodding	561	SY		
174	SP	Sand Aggregate Material for Sand Filter BMP	997	TN		
175	1610	Fence, 5' Chain Link Fence	280	LF		
176	876	Fence Removal. 5' Chain Link Fence	300	LF		
177	240	Fence, 5' Chain Link (Double Gate)	1	EA		
178	852	Tree Removal	28	EA		
179	610	Tree - 3" Caliper	28	EA		
180	SP	AIRCRAFT DISPLAY CONCRETE FOUNDATION	1	LS		
181	SP	BOLLARD AND CHAIN BARRIER	26	EA		

		ADDITIVE BID ALTERNATES			
182	1665	SYNTHETIC TURF AT MOCK RUNWAY	3,730	SF	
183	815	6" PVC SUBDRAIN, SCHEDULE 80	200	LF	
184	520	AGGREGATE BASE COURSE	140	TN	
		UNIT PF			

In case of error in extension of prices in the Bid, the unit prices, where available, shall govern.

BID GUARANTEE

The undersigned Bidder agrees to execute the Contract for the above amount and to furnish surety as specified within 10 days after notice of award, if offered within 120 calendar days after receipt of bids, and upon failure to do so agrees to forfeit the attached cash, cashier's check, certified check, U. S. money order, or bid bond, as liquidated damages for such failure, in the amount of:

___ Dollars (\$______)

the stated amount constituting five percent (5%) of the Base Bid amount above.

BID SUPPLEMENTS

Attached to this Bid Form and incorporated herein are the following documents, completed in full by the undersigned:

Certificate of Non-Discrimination DBE Form # 3 DBE Form # 5 Bid Bond Buy American Certification Tax Deficiency and Felony Convictions Certification Lobbying and Influencing Federal Employees Certification

Form copies of DBE Form #3 and DBE Form #5 can be found at:

http://www.cltairport.com/doingbusinesswithCLT/pages.default.aspx

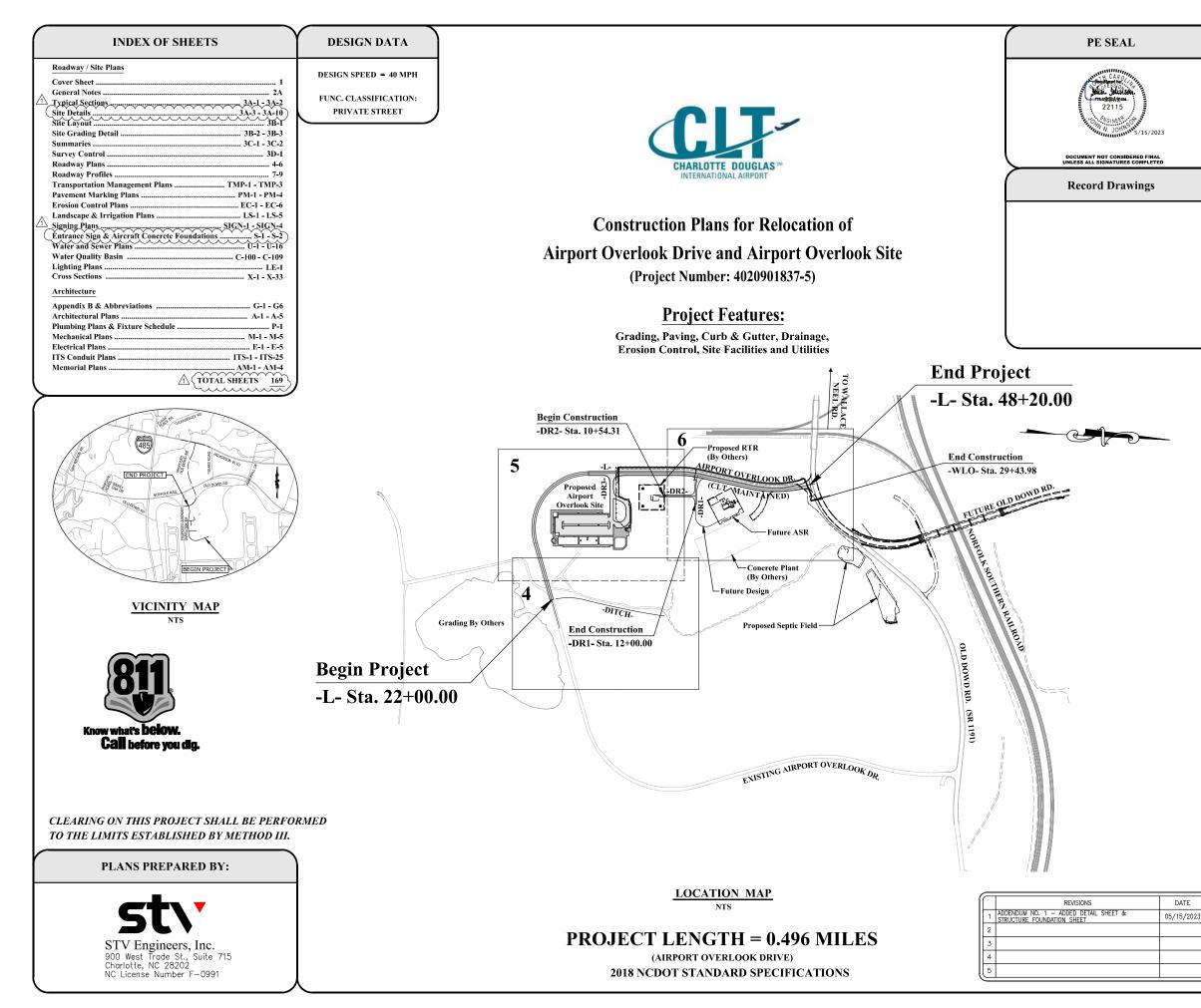
PLEASE NOTE - FAILURE TO INCLUDE ALL BID SUPPLEMENTS MAY RESULT REJECTION OF THIS BID.

CONTRACTOR'S LICENSE

The undersigned further states that he is a duly licensed Contractor, for the type of work proposed, in the State of North Carolina, and that all fees, permits, etc., pursuant to the submission of this proposal have been paid in full. LICENSE #

CONFIDENTIALITY REQUIREMENTS

By signing this bid form, I acknowledge that I have read, understand and shall comply with the confidentiality requirements as stated in the Instruction to Bidders, Section 13.



CONVENTIONAL SYMBOLS

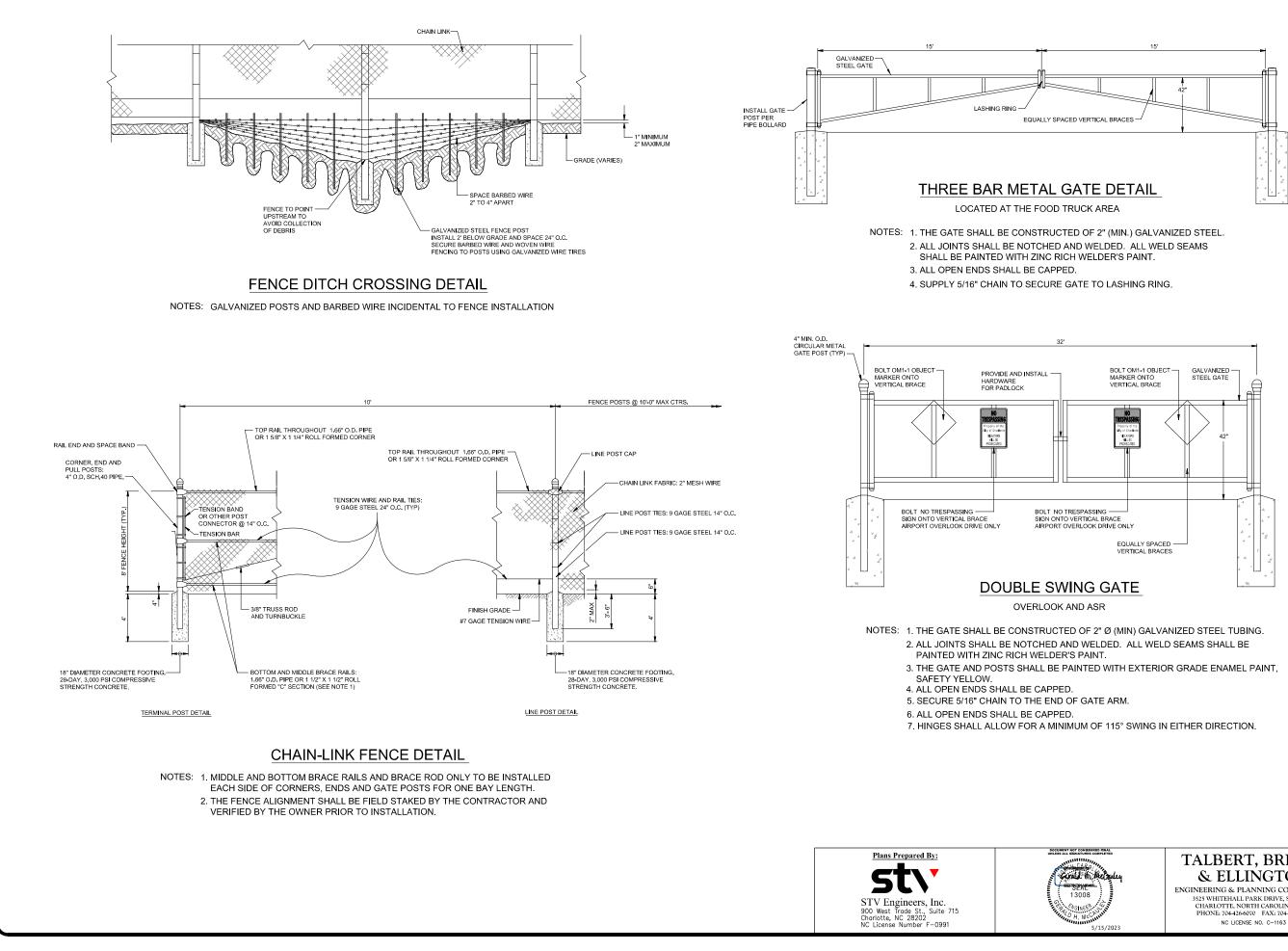
CONVENTIONAL SYMBOLS
New Property Line
Existing Property Line
Maintained as R/W Line
Existing Structures
Railroad Tracksx
Slope Stake Line
Limits of Disturbance Line LOD LOD LOD
Limits of Construction Line LOC LOC LOC
Temporary Construction Easement e e e e
Lease Easement E
Slope Easement
Drainage Utility Easement DUE
Utility Easement PUE
Post Construction Controls Easement
Conservation Easement
Existing Gas Eline
Proposed Water Line w
Existing Sanitary Sewer ss ss ss ss
Existing Underground Telecommunications — or — or — or —
Existing Underground Electric ut
Proposed Storm Drainage
Accessible Ramp
Existing Tree
Existing Water Meter
Existing Water Valve
Existing Gas Valve
Existing Sanitary Sewer Manhole
Proposed Sanitary Sever Manhole
Existing Storm Drain Manhole
Proposed Storm Drain Manhole
Existing Telephone Manhole
Proposed Telephone Manhole
Existing Electric Manhole
Proposed Electric Manhole
Existing Catch Basin
Proposed Catch Basin
Existing Light Pole
Proposed Light Pole
Existing Utility Pole
Guy Wire
Proposed Utility Pole
-
Existing Fire Hydrant
Proposed Fire Hydrant
Existing Drop Inlet
Proposed Drop Inlet
Tree Protection
Proposed Guardrail
Silt Fence
Proposed Curb & Gutter, Conc. Drive, Sidewalk
Proposed Asphalt Pavement
Proposed Rip Rap Ditch
Proposed Gravel
Proposed Pavement Removal
Asphalt Milling
/
GRAPHIC SCALES

Plan View	40	80	120
Horz. Profile	40	80	120
Vert. Profile	8	16	24
	5	10	15
Cross Section			

100% Plan Submittal

05/15/2023

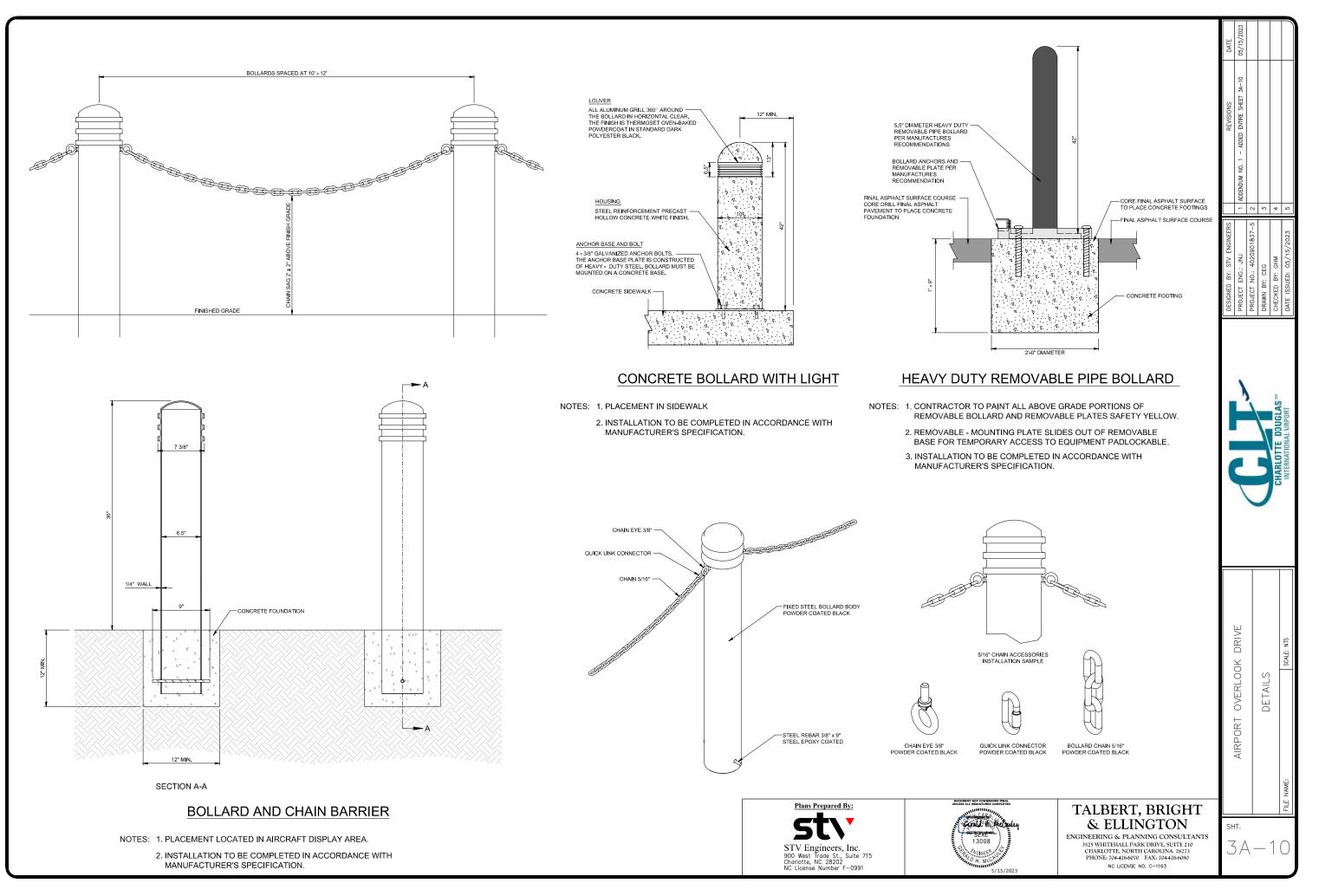
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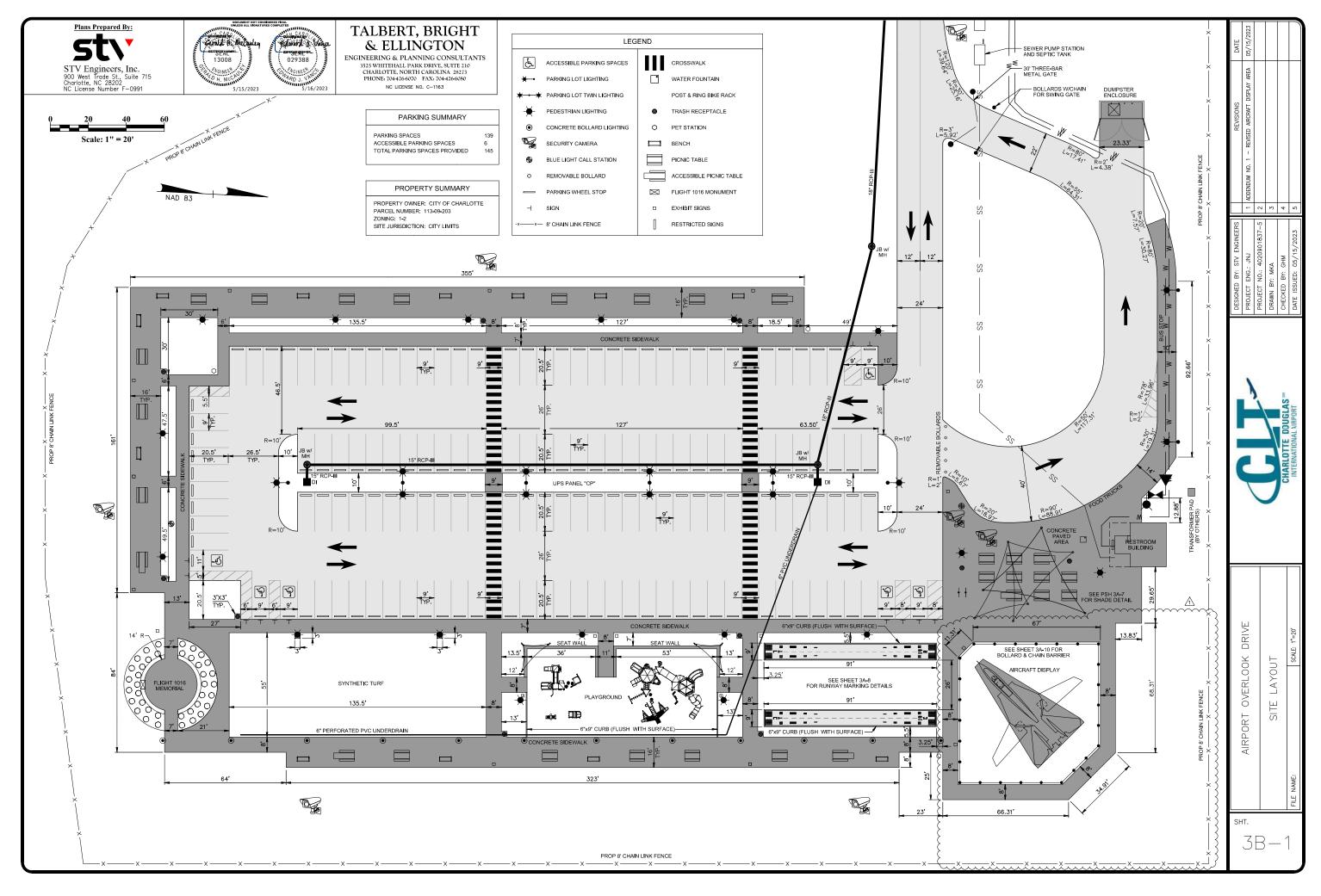


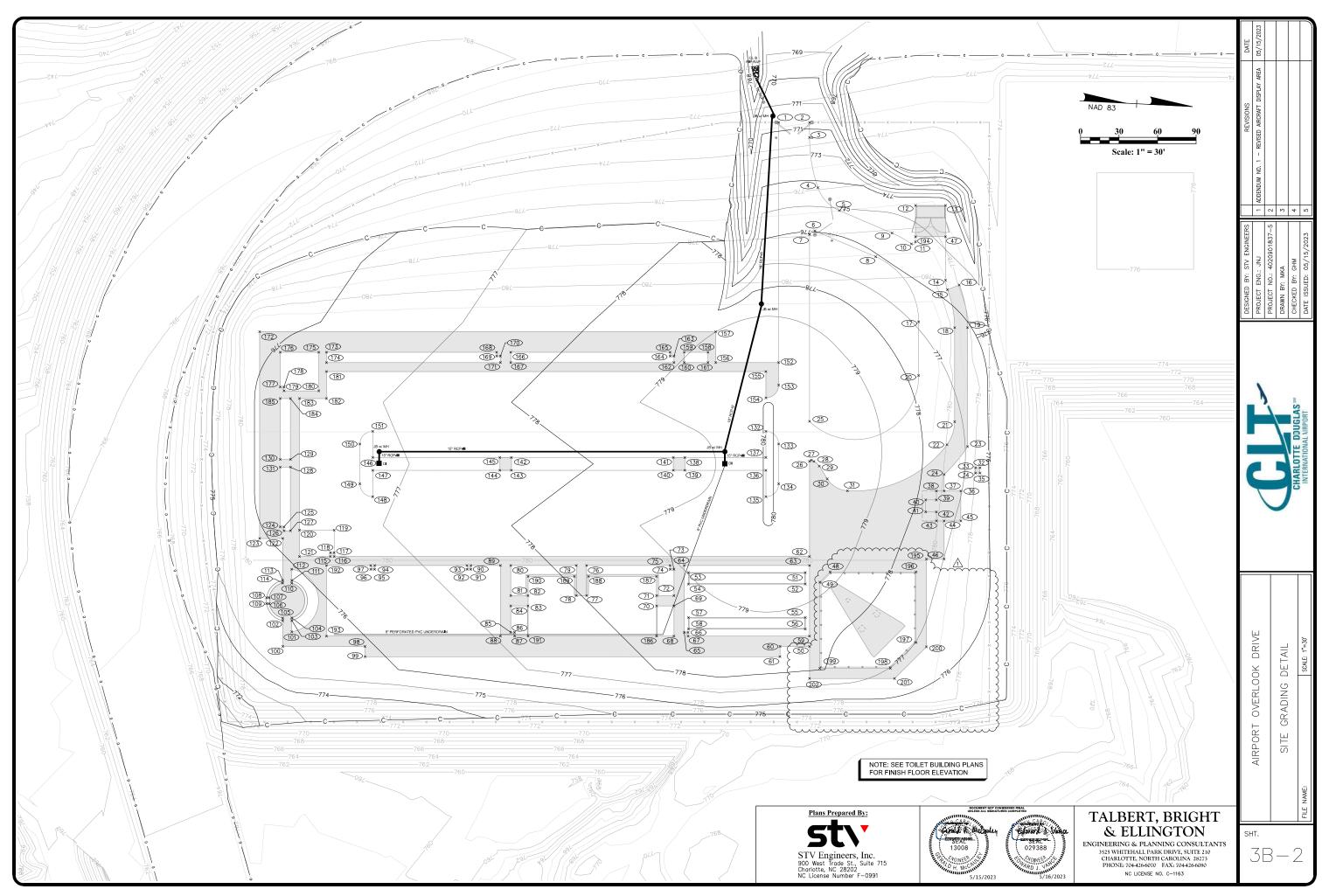
TALBERT, BRIGHT & ELLINGTON

ENGINEERING & PLANNING CONSULTANTS 3525 WHITEHALL PARK DRIVE, SUITE 210 CHARLOTTE, NORTH CAROLINA 28273 PHONE: 704-426-6070 FAX: 704-426-6080 NC LICENSE NO. C-1163

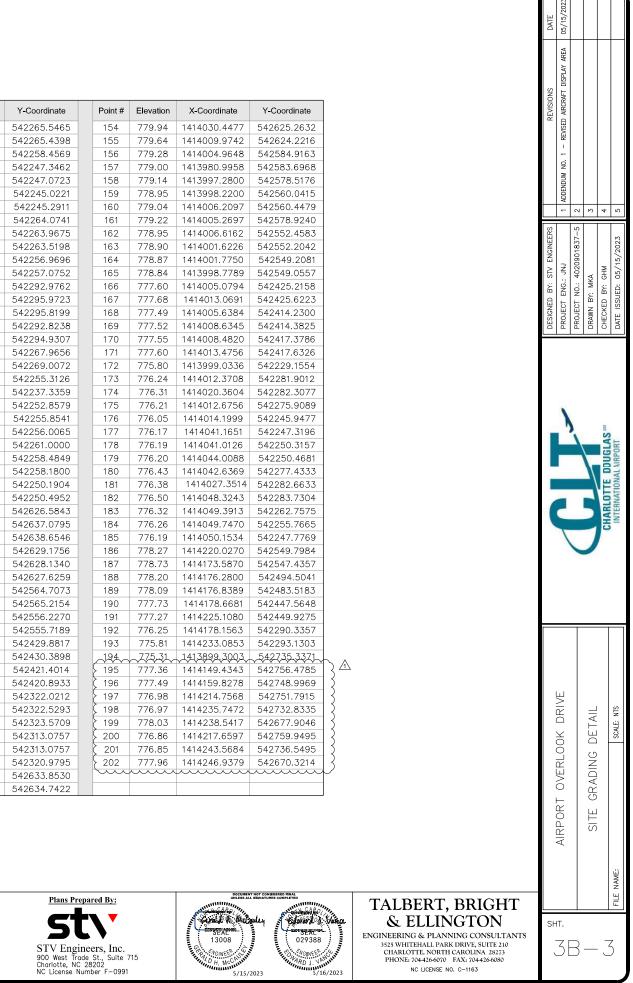
DESIGNED BY: STV ENGINEERS REVISIONS DATE	PROJECT ENG: JNJ 1 PLACED ON SHEFT 34-10 DETAILS & 165/15/2023	PROJECT NO.: 4020901837-5 2	DRAWN BY: CEC 3	CHARLOTTE DUIG AS W CHECKED BY: GHM 4	INTERNATIONAL AIRPORT DATE ISSUED: 05/15/2023 5			
AIRPORT OVERLOOK DRIVE DETAILS scale name: scale ints								
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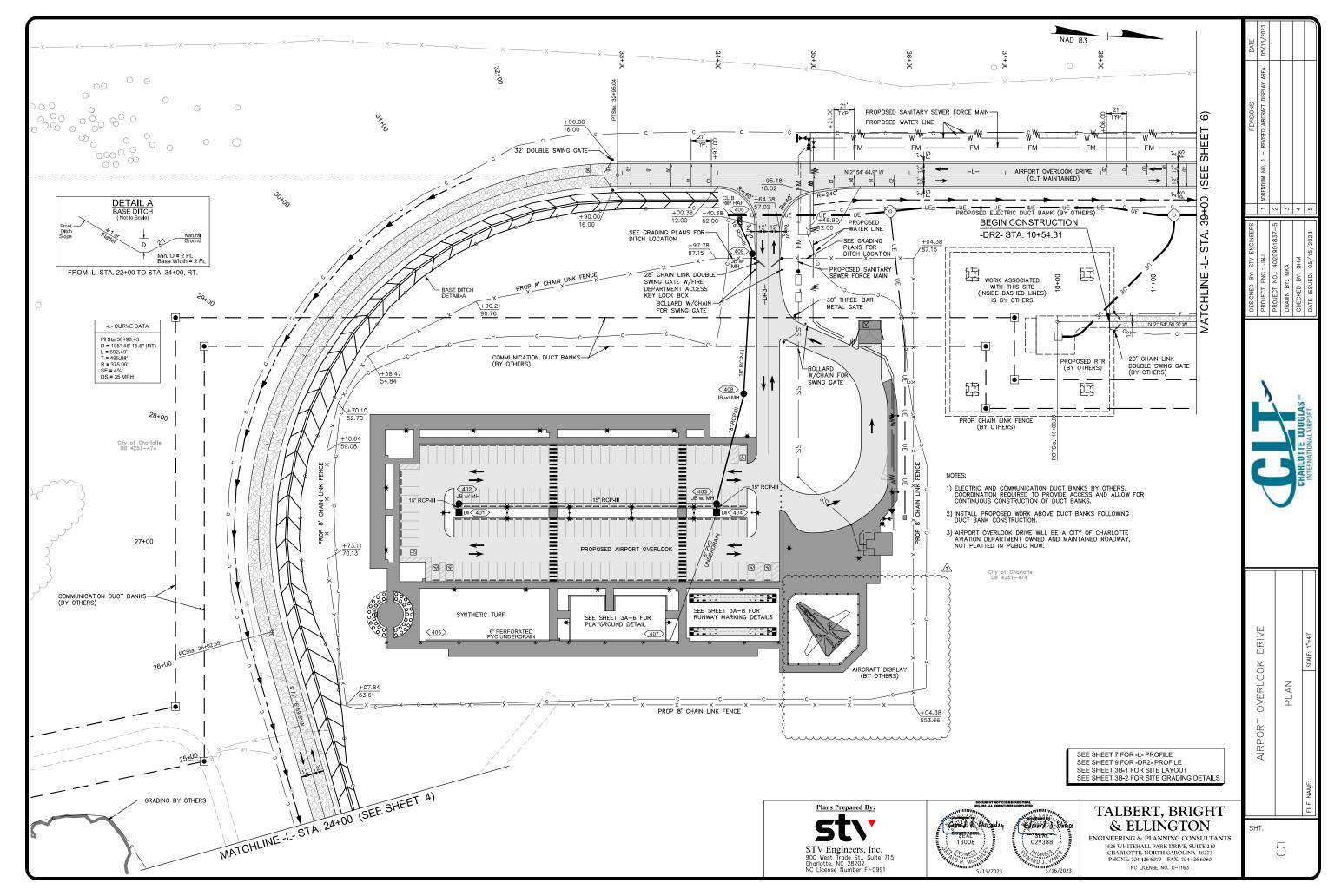


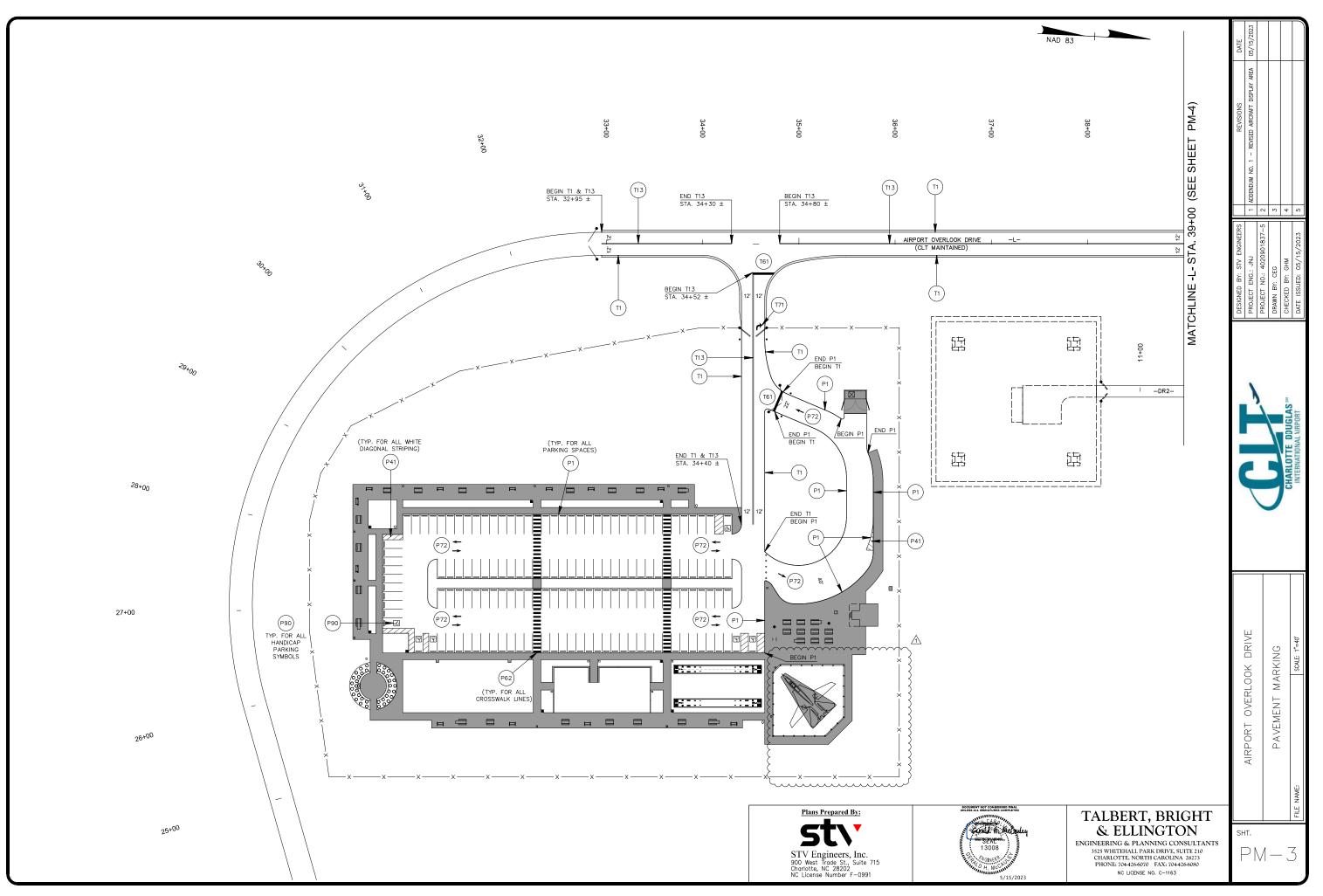


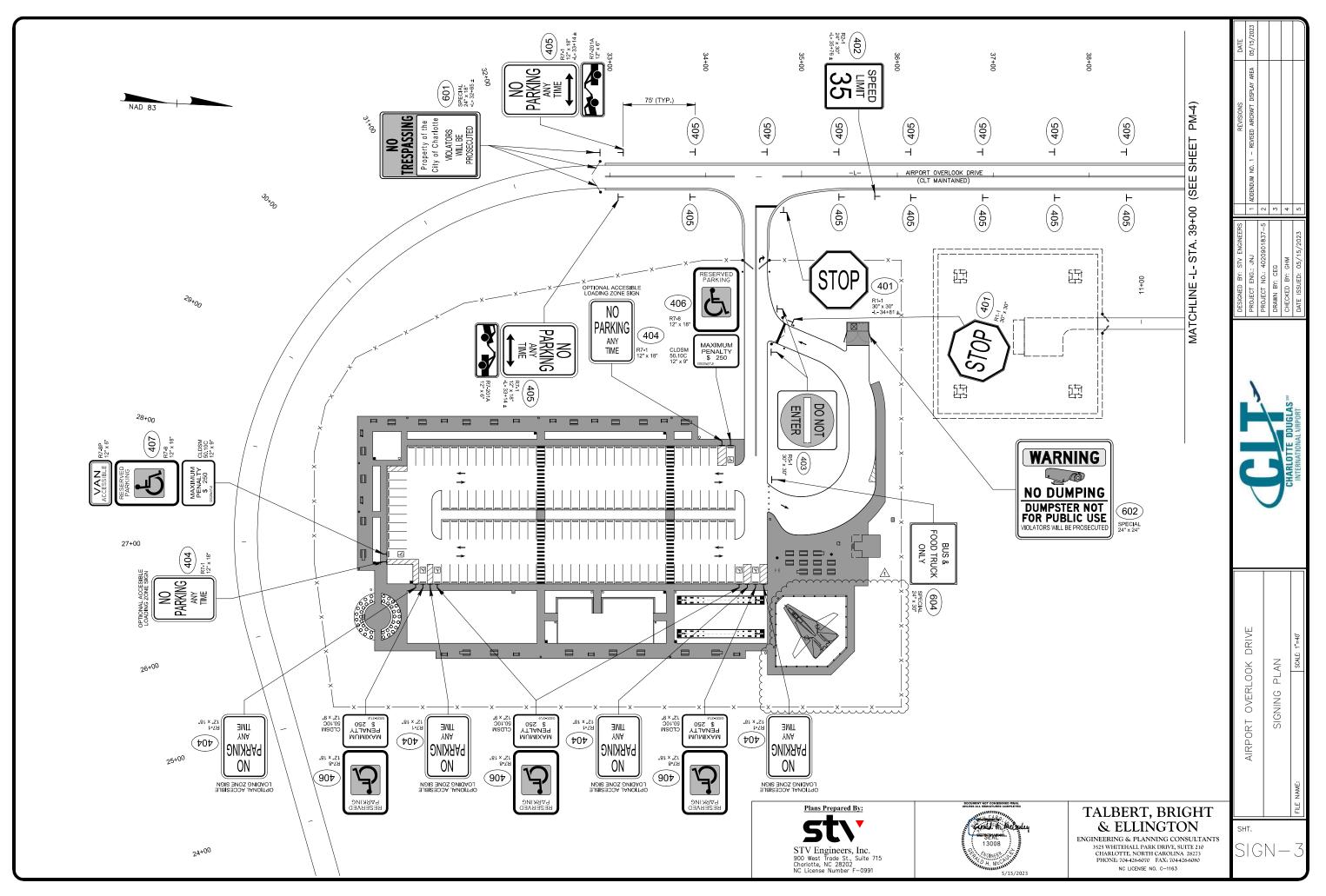


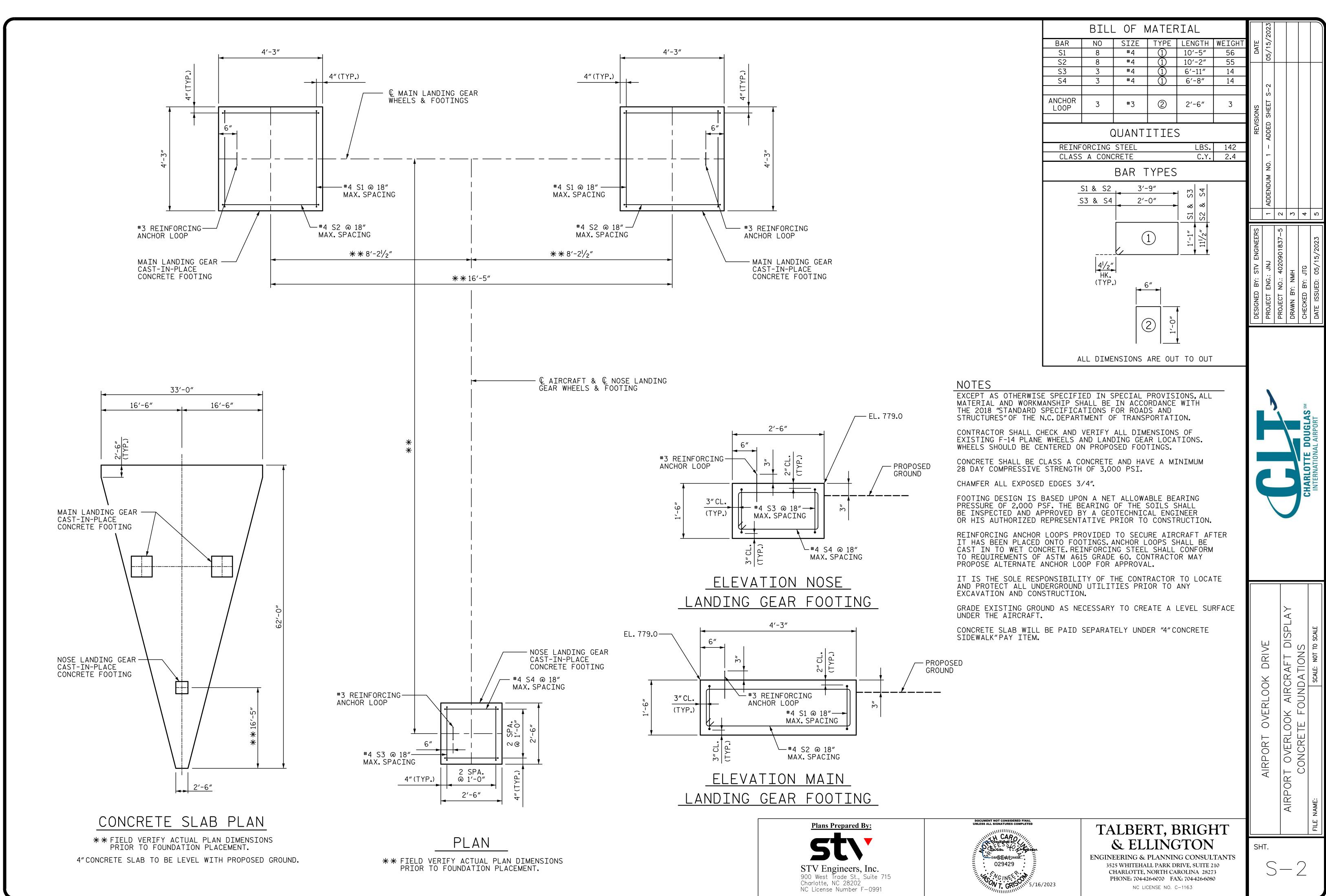
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1	771.66	1413815.8659	542624.3590	52	779.21	1414173.6980	542663.3410	103	775.74	1414222.2966	542265.5465	154	779.94	1414030.4477	542625
2	771.74	1413814.6464	542648.3280	53	779.01	1414169.3333	542572.0013	104	775.75	1414220.1998	542265.4398	155	779.64	1414009.9742	542624
3	772.24	1413826.3986	542648.9259	54	778.92	1414178.3217	542572.4586	105	775.71	1414220.7131	542258.4569	156	779.28	1414004.9648	542584
4	774.26	1413864.9500	542657.3865	55	778.74	1414199.6644	542664.6621	106	775.70	1414210.1059	542247.3462	157	779.00	1413980.9958	542583
5	775.09	1413881.9613	542674.9244	56	778.58	1414208.6528	542665.1194	107	775.72	1414205.4563	542247.0723	158	779.14	1413997.2800	542578
6	775.95	1413898.7507	542656.7850	57	778.66	1414204.2881	542573.7796	108	775.71	1414205.5606	542245.0221	159	778.95	1413998.2200	542560
7	776.07	1413901.7205	542652.7580	58	778.57	1414213.2765	542574.2369	109	775.69	1414210.2105	542245.2911	160	779.04	1414006.2097	542560
8	776.09	1413916.4177	542705.0708	59	778.43	1414213.9805	542668.6447	110	775.90	1414193.3560	542264.0741	161	779.22	1414005.2697	542578
9	775.53	1413897.3149	542716.8874	60	778.53	1414223.1388	542646.0808	111	775.92	1414191.2594	542263.9675	162	778.95	1414006.6162	542552
10	775.51	1413905.0240	542732.4634	61	778.39	1414231.1285	542646.4873	112	775.96	1414182.4608	542263.5198	163	778.90	1414001.6226	542552
11	775.41	1413903.2369	542735.5374	62	779.42	1414152.0606	542665.4944	113	775.87	1414191.4786	542256.9696	164	778.87	1414001.7750	542549
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38	777.59	1414096.1245	542761.6052	89	777.60	1414171.2715	542425.6607	140	778.50	1414090.5331	542556.2270	191	777.27	1414225.1080	542449
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43	777.41	1414119.4277	542762.7908	94	776.63	1414176.2509	542327.7873	145	777.53	1414087.4054	542420.8933	(196	777.49	1414159.8278	542748
44	777.23	1414119.1228	542768.7830	95	776.60	1414179.2471	542327.9397	146	776.80	1414092.4357	542322.0212	> 197	776.98	1414214.7568	542751
45	776.81	1414118.4623	542781.7662	96	776.57	1414179.3995	542324.9436	147	776.82	1414102.4227	542322.5293	¢ 198	776.97	1414235.7472	542732
46	776.99	1414148.7316	542770.2894	97	776.60	1414176.4034	542324.7912	148	776.87	1414122.8963		2 199	778.03	1414238.5417	542677
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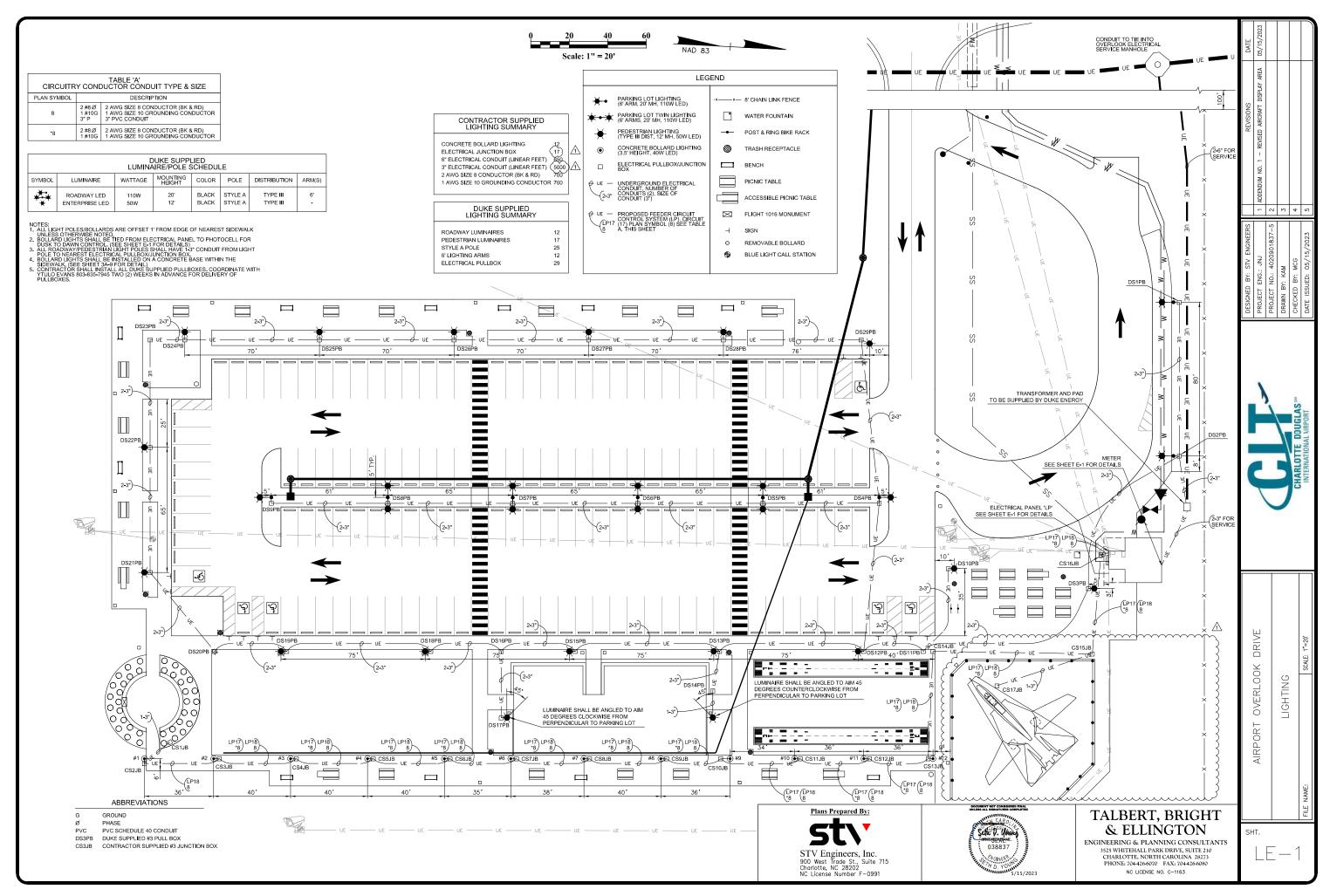


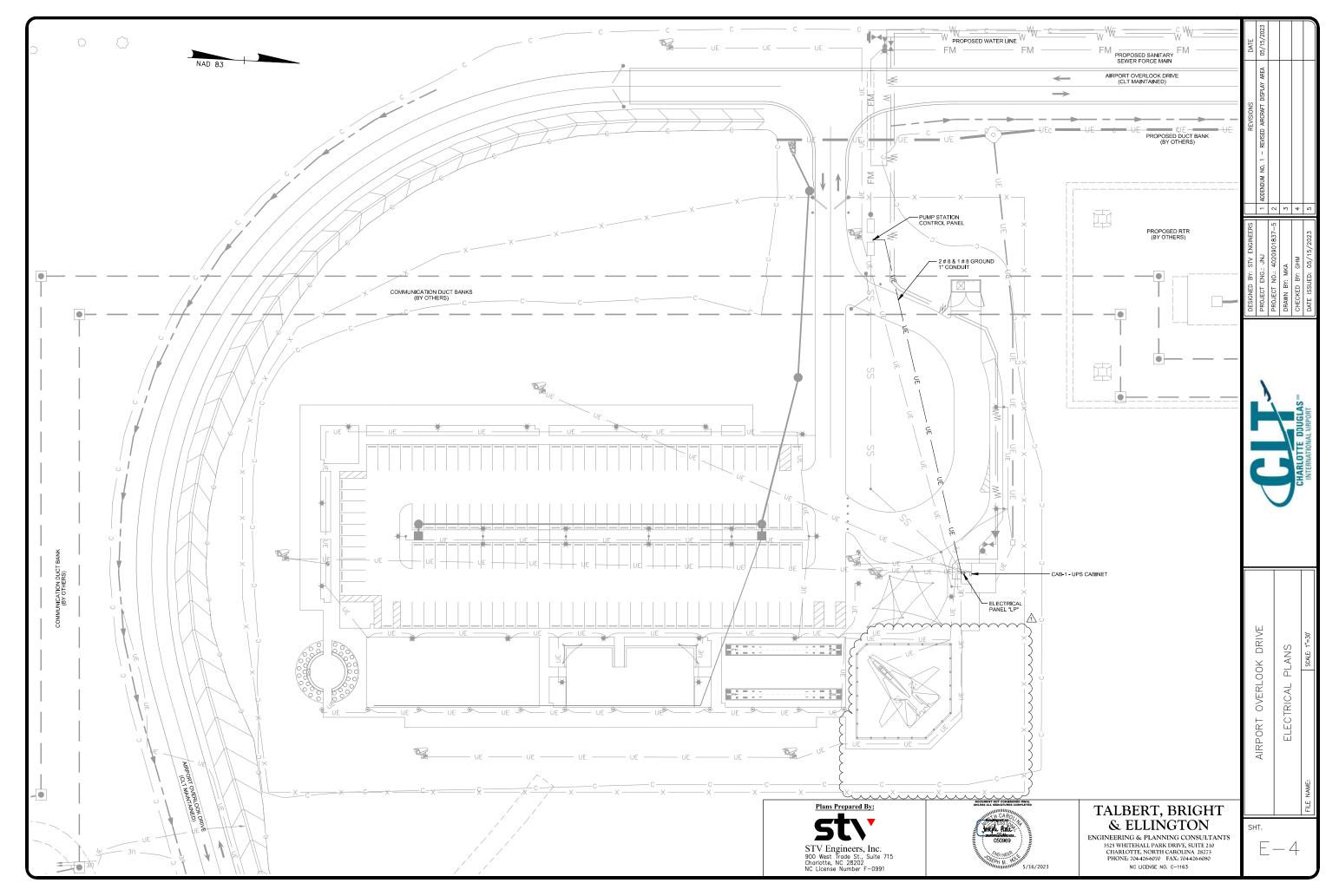


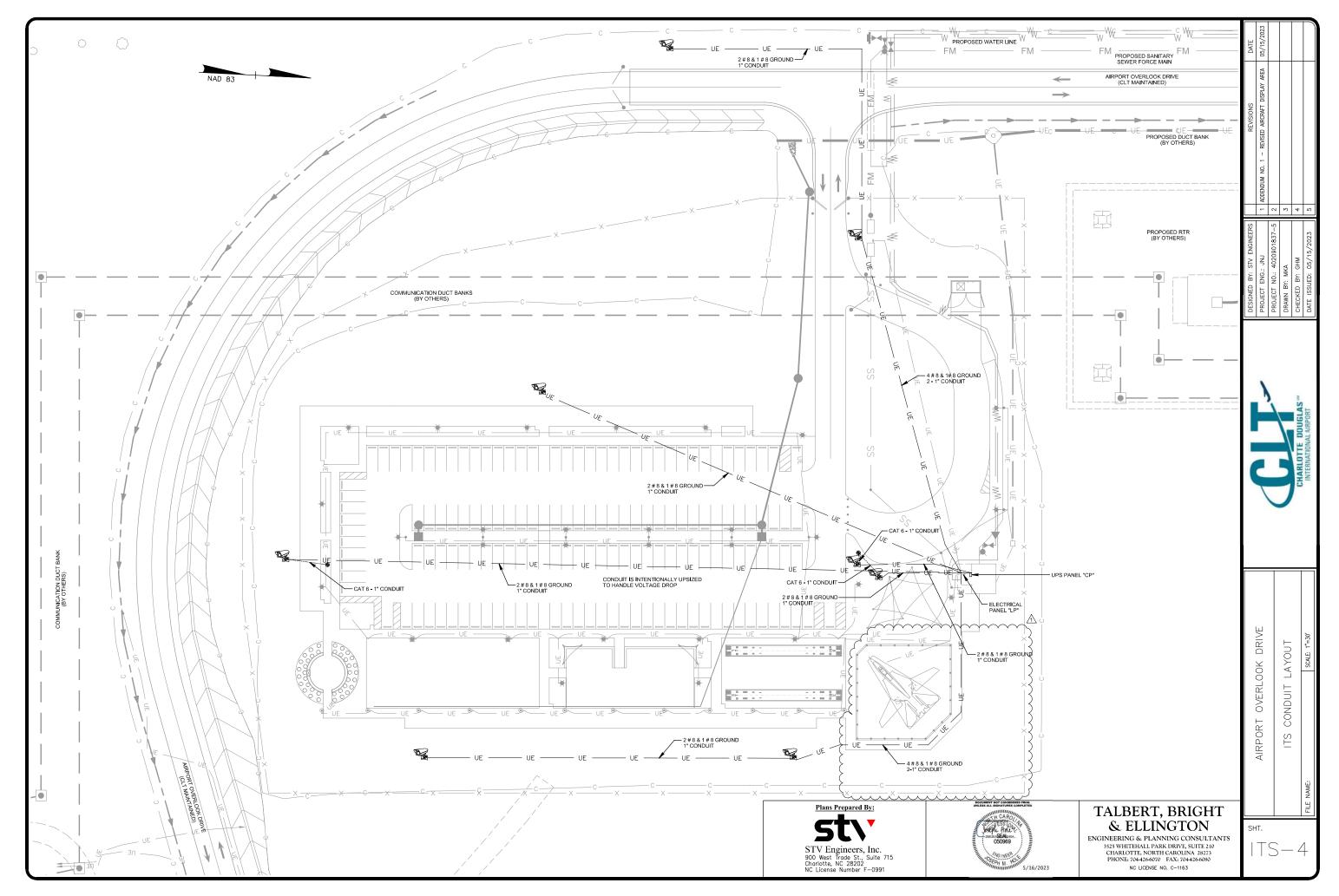


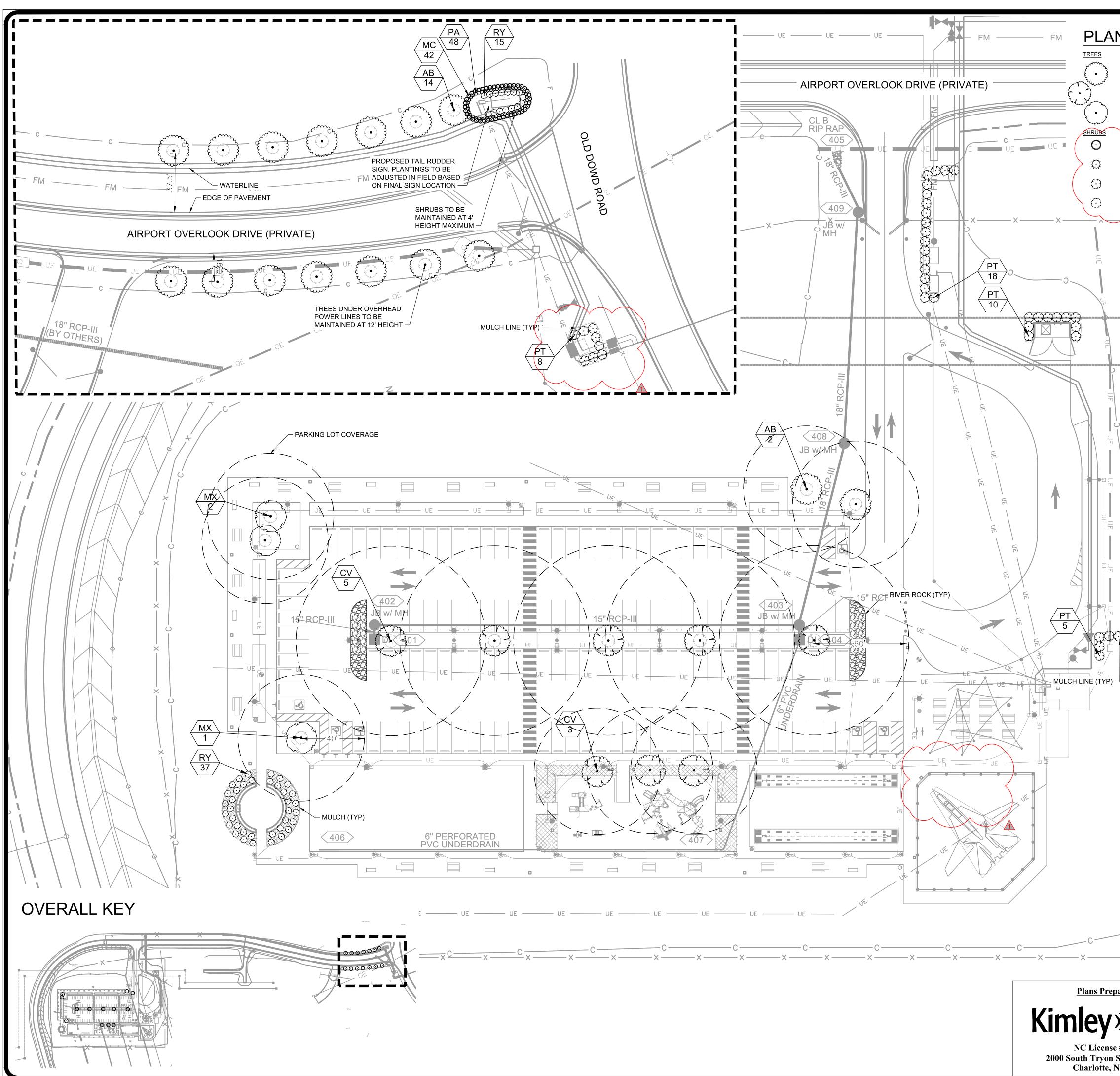








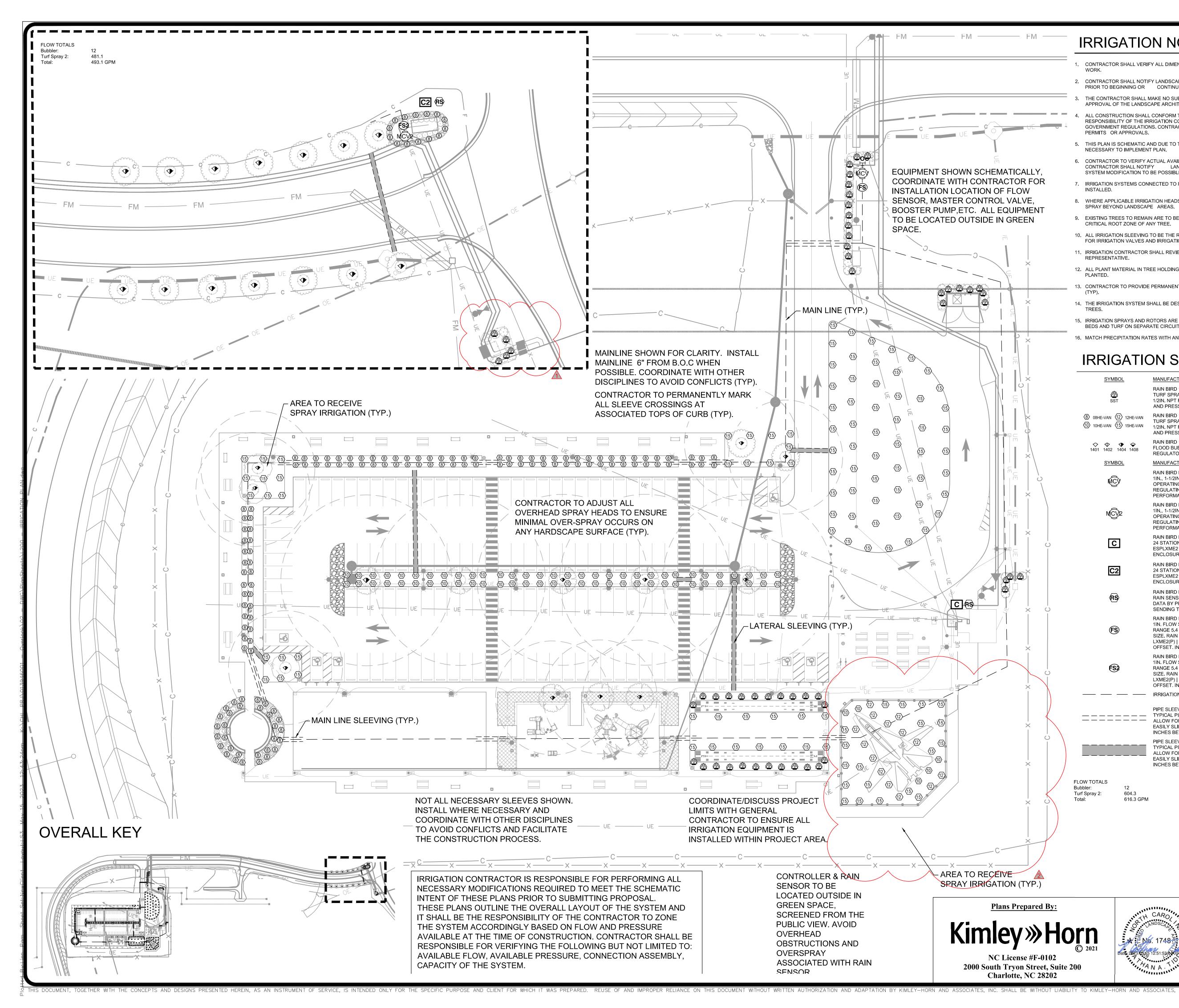




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THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY KIMLEY-HORN AND ASSOCIATES, INC. SHALL

	SCF	HEDUL	F							23			
				BOTANICAL NAME		METHOD	SIZE	CAL	DATE	05/02/2023			
AB	16	TRIDENT MAPL		ACER BUERGERIAN	UM	F.G., B & B	8` MIN HEIGHT	2" MIN CAL.		FOR			
CV	8	SINGLE, STRAIC WHITE FRINGE SINGLE, STRAIC	TREE	CHIONANTHUS VIRG	BINICUS	F.G., B & B	8` MIN HEIGHT	2" MIN CAL.		ENTS FC			
MX	3	JANE MAGNOLI SINGLE, STRAIG	A	MAGNOLIA X 'JANE'		F.G., B & B	8` MIN HEIGHT	2" MIN CAL.	ر س	SETBACK REQUIREMENTS			
				BOTANICAL NAME		CONTAINER	SPACING	SIZE	REVISIONS	ACK REC			
MC	42	PINK MUHLY GF FULL	RASS	MUHLENBERGIA CAF	PILLARIS	3 GAL.	36" O.C.	30" FULL	REV	R SETB/			
PA	48	CASSIAN FOUN FULL	TAINGRASS	PENNISETUM ALOPE	ECUROIDES 'CASSIAN'	3 GAL.	30" O.C.	24" FULL		TED PEI /ENTER			
PT	41	JAPANESE PITT FULL	OSPORUM	PITTOSPORUM TOBI	IRA	3 GAL.	36" O.C.	30" HT MIN		W PREV			
RY	52	YEDDA HAWTH		RHAPHIOLEPIS UMB	ELLATA	3 GAL.	36" O.C.	30" FULL		LANDSCAPING ADJUSTED PER THE BACKFLOW PREVENTER			
		MULCH LINE								LANDS THE B/			
>		MULCH								-	N M) 4	വ
		RIVER ROCK											
>	MISC.	<u>QTY</u> <u>BOTANIC</u>	AL/COMMON N/	AME	SPECIFICATIONS				PROF 1	PROF 1	PROJ #		2022
0.00000	MULCH		TUS MULCH			M, SHREDDED, F	REE OF WEEDS/IN	/ASIVE	LIC PR		A D		9/30/202
 ×	SOD	TBD BERMUD			ROLLED TIGHT, 10	00% WEED/INSEC	CT/DISEASE FREE		BY:	ENG.: LIC		÷.	
				BE PLANTED WITH					DESIGNED		ب ش	1 1.1	E ISSUED:
	BE			BE PLANTED WITH CIFICATIONS FOR I					DESI	PRO		CHEC	DATE
										1 1		1	
	SH PL	HREDDED HAR AN FOR MULC	DWOOD MU H BOUNDAF		APE								
	LA			YPICAL MULCH AR	ξEA								
	חאסענ		NTS (ner city	code section 21.96)			7						
K J O	Parkin			Required) Provid	ed	1						
	Trees			space may be more 40' from a tree	No: parking tree limited to provide v		d			١			
×	INTER	NAL PLANTING F		TTS (per city code se						')	1	AS sw	
	Imperv	vious cover square feet		Required	Provid	ed	1					AIRPOR	
- × 		ape area		andscape area .000 sf. = 10 trees	9,460 sq 13	ı. ft.	1					TTE D	
	TREE T	YPE REQUIREMI uired trees	ENTS (per city	y code section 21.96 Required		ed	-					HARLOTTE DOUGLAS INTERNATIONAL AIRPORT	
 × 		naturing trees		% = 10 trees	Tree heights are li airfield regu	imited due to	1					CHA	
		species		% = 7 trees ci ty code section 21.	13 tree								
 	Trees	Drive (200 lf)	F	Required ees per side	.96) Provid 7 trees pe		-						
Ť ,		\frown		$\frown \frown \frown$	3 BECAUSE PARKIN	\checkmark							
	NOT V	ISIBLE FROM A	DJACENT P	ROPERTIES OR RO	DW. THE PARKING A W (OLD DOWD ROAI	AREA IS							
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pared D	HO 102		H H H H H H H H H H H H H H H H H H H	CAROL $ANDSCASCASCASCASCASCASCASCASCASCASCASCASCAS$	ENGINEERING 3525 WHIT CHARLOT	0 15 ERT, ELLIN G & PLANN EHALL PARK	BRIGI GTON ING CONSUI DRIVE, SUITE CAROLINA 282	60 HT LTANTS 210 73	SH		S.	-1	ليا
pared 1	H0 102 c, Suite 2		L L L L L L L L L L L L L L L L L L L	CARO $ANDSC43$ $CARO$ $ANDSC43$	ENGINEERING 3525 WHIT CHARLOT PHONE: 7	0 15 ERT, ELLIN G & PLANN EHALL PARK	BRIGI GTON ING CONSUI DRIVE, SUITE CAROLINA 282 FAX: 704-426-608	60 HT LTANTS 210 73	SH		S-	-1	ليا



	RRIGATIC	ON NOTES		
1.	CONTRACTOR SHALL VERI WORK.	FY ALL DIMENSIONS, ELEVATIONS, EQUIPMENT QUANTITIES, ETC. PRIOR TO	D BEGINNING	
2.	CONTRACTOR SHALL NOTI PRIOR TO BEGINNING OR	FY LANDSCAPE ARCHITECT OF ANY DISCREPANCIES IN PLANS OR SPECIFIC CONTINUING WORK.	CATIONS	
3.	THE CONTRACTOR SHALL APPROVAL OF THE LANDS	MAKE NO SUBSTITUTIONS, DELETIONS, OR ADDITIONS TO THIS PLAN WITH CAPE ARCHITECT.	OUT	
4.	RESPONSIBILITY OF THE IF	L CONFORM TO CITY, COUNTY, STATE, AND FEDERAL REQUIREMENTS. IT S REIGATION CONTRACTOR TO ENSURE THAT ALL IRRIGATION EQUIPMENT M INS. CONTRACTOR SHALL ALSO BE RESPONSIBLE FOR OBTAINING ANY NEC	EETS	
5.	THIS PLAN IS SCHEMATIC / NECESSARY TO IMPLEMEN	AND DUE TO THE NATURE OF CONSTRUCTION SLIGHT FIELD MODIFICATION IT PLAN.	IS MAY BE	
6.	CONTRACTOR TO VERIFY A CONTRACTOR SHALL NOTI SYSTEM MODIFICATION TO			
7.	IRRIGATION SYSTEMS CON INSTALLED.	INECTED TO POTABLE WATER SUPPLY, SHALL HAVE A BACKFLOW PREVEN	TER	
8.	WHERE APPLICABLE IRRIG SPRAY BEYOND LANDSCAI	ATION HEADS ARE TO BE ADJUSTED FOR COMPLETE COVERAGE WITH MIN PE AREAS.	IMUM OVER	
9.	EXISTING TREES TO REMA CRITICAL ROOT ZONE OF A	IN ARE TO BE PROTECTED FROM DAMAGE. DO NOT TRENCH OR EXCAVATE ANY TREE.	E WITHIN THE	
10.		TO BE THE RESPONSIBILITY OF THE IRRIGATION CONTRACTOR. ELECTRIC	CAL WIRES	
11.	IRRIGATION CONTRACTOR REPRESENTATIVE.	SHALL REVIEW WINTERIZATION PROCEDURES FOR IRRIGATION SYSTEM W	/ITH OWNER'S	6
12.	ALL PLANT MATERIAL IN TE PLANTED.	REE HOLDING AREAS SHALL BE MANUALLY WATERED/IRRIGATED TO KEEP I	MOIST UNTIL	
13.	CONTRACTOR TO PROVIDE (TYP).	E PERMANENT BENCH-MARKS ON ALL CURB LINES AT RELATED SLEEVE LC	CATIONS	
14.	THE IRRIGATION SYSTEM STREES.	SHALL BE DESIGNED AND INSTALLED TO MINIMIZE ROOT DISTURBANCE IN E	EXISTING	
15.	IRRIGATION SPRAYS AND F BEDS AND TURF ON SEPAF	ROTORS ARE NOT COMBINED ON THE SAME CONTROL VALVE CIRCUIT - LAN RATE CIRCUITS.	IDSCAPE	
16.	MATCH PRECIPITATION RA	TES WITH ANY HEADS THAT ARE REPLACED.		
	IRRIGATIO	ON SCHEDULE		
	<u>SYMBOL</u>	MANUFACTURER/MODEL/DESCRIPTION	<u>QTY</u>	PSI
	A SST	RAIN BIRD 1806-U-SAM-PRS 15 STRIP SERIES TURF SPRAY 6IN. POP-UP SPRINKLER WITH CO-MOLDED WIPER SEAL. 1/2IN. NPT FEMALE THREADED INLET. WITH SEAL-A-MATIC CHECK VALVE AND PRESSURE REGULATING.	54 ,	30
	(8) 08HE-VAN (12) 12HE-VAN	RAIN BIRD 1806-U-SAM-PRS HE-VAN SERIES TURF SPRAY 6IN, POP-UP SPRINKLER WITH CO-MOLDED WIPER SEAL. 1/2IN, NPT FEMALE THREADED INLET. WITH SEAL-A-MATIC CHECK VALVE	351	30
	10 10HE-VAN (15) 15HE-VAN	AND PRESSURE REGULATING.		
	 (10) 10HE-VAN (15) 15HE-VAN (15) 15		26	20

RAIN BIRD PESB-PRS-D 1" 1IN., 1-1/2IN., 2IN. PLASTIC INDUSTRIAL MASTER VALVES. LOW FLOW OPERATING CAPABILITY, GLOBE CONFIGURATION. WITH PRESSURE REGULATING MODULE, AND SCRUBBER TECHNOLOGY FOR RELIABLE PERFORMANCE IN DIRTY WATER IRRIGATION APPLICATIONS. RAIN BIRD PESB-PRS-D 1" 1IN., 1-1/2IN., 2IN. PLASTIC INDUSTRIAL MASTER VALVES. LOW FLOW OPERATING CAPABILITY, GLOBE CONFIGURATION. WITH PRESSURE REGULATING MODULE, AND SCRUBBER TECHNOLOGY FOR RELIABLE PERFORMANCE IN DIRTY WATER IRRIGATION APPLICATIONS. RAIN BIRD ESPLXME2 W/ (1) ESPLXMSM12 С 24 STATION, TRADITIONALLY-WIRED, COMMERCIAL CONTROLLER. (1) ESPLXME2 12-STATION, INDOOR/OUTDOOR, PLASTIC WALL-MOUNT ENCLOSURE W/ (1) ESPLXMSM12 - 12-STATION EXPANSION MODULES. RAIN BIRD ESPLXME2 W/ (1) ESPLXMSM12 C2 24 STATION, TRADITIONALLY-WIRED, COMMERCIAL CONTROLLER. (1) ESPLXME2 12-STATION, INDOOR/OUTDOOR, PLASTIC WALL-MOUNT ENCLOSURE W/ (1) ESPLXMSM12 - 12-STATION EXPANSION MODULES. RAIN BIRD RAINGAUGE RAIN SENSOR FOR MAXICOM OR SITECONTROL. CUSTOMIZES WEATHER 2 DATA BY PROVIDING SITE-SPECIFIC RAINFALL MEASUREMENTS, AND SENDING TO CENTRAL CONTROLLER DAILY. RAIN BIRD FS-100-P (2) 1IN. FLOW SENSOR, PLASTIC PVC MODEL, SUGGESTED OPERATING (FS) RANGE 5.4 GPM TO 54 GPM. SIZE FOR FLOW NOT ACCORDING TO PIPE SIZE. RAIN BIRD COMPATIBLE CONTROLLERS: ESP-LXIVM(P) | LXD | LXME2(P) | ME3, OR CONTROLLERS ACCEPTING CUSTOM K FACTOR AND OFFSET INSTALL IN RAIN BIRD VALVE BOX RAIN BIRD FS-100-P (2) 1IN. FLOW SENSOR, PLASTIC PVC MODEL. SUGGESTED OPERATING FS2 RANGE 5.4 GPM TO 54 GPM. SIZE FOR FLOW NOT ACCORDING TO PIPE SIZE. RAIN BIRD COMPATIBLE CONTROLLERS: ESP-LXIVM(P) | LXD | LXME2(P) | ME3, OR CONTROLLERS ACCEPTING CUSTOM K-FACTOR AND OFFSET. INSTALL IN RAIN BIRD VALVE BOX ----- IRRIGATION MAINLINE: PVC SCHEDULE 40 1,775 L.F. PIPE SLEEVE: PVC CLASS 200 _____ ___ ___ ___ TYPICAL PIPE SLEEVE FOR IRRIGATION PIPE. PIPE SLEEVE SIZE SHALL 214.7 L.F. ALLOW FOR IRRIGATION PIPING AND THEIR RELATED COUPLINGS TO
EASILY SLIDE THROUGH SLEEVING MATERIAL. EXTEND SLEEVES 18 INCHES BEYOND EDGES OF PAVING OR CONSTRUCTION. PIPE SLEEVE: PVC CLASS 200 TYPICAL PIPE SLEEVE FOR IRRIGATION PIPE. PIPE SLEEVE SIZE SHALL ALLOW FOR IRRIGATION PIPING AND THEIR RELATED COUPLINGS TO EASILY SLIDE THROUGH SLEEVING MATERIAL. EXTEND SLEEVES 18 274.3 L.F.

INCHES BEYOND EDGES OF PAVING OR CONSTRUCTION. FLOW TOTALS Bubbler: Turf Spray 2: 604.3 616.3 GPM Total:





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